WATER-RESOURCES APPRAISAL OF
THE WET MOUNTAIN VALLEY, IN
FARTS OF CUSTER AND FREMONT COUNTIES, COLORADO
U. S. GEOLOGICAL SURVEY


Water-Resources Investigations 78-1

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Prepared in cooperation with the
Southeastern Colorado Wateroloradg wath
Conservancy District



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UNITED STATES
DEPARTMENT OF THE INTERIOR
GEOLOGICAL SURVEY

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IN PARTS OF CUSTER AND FREMONT COUNTIES, COLORADO
By Clark J. Londquist and Russe11 K. Livingston
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# UNITED STATES DEPARTMENT OF THE INTERIOR <br> CECIL D. ANDRUS, Secretary <br> GEOLOGICAL SURVEY <br> W. A. Radlinski, Acting Director 

Page
Metric conversion ..... IV
Abstract. ..... 1
Introduction. ..... 2
Geohydrologic setting ..... 2
Ground-water occurrence ..... 6
Mountainous area ..... 6
Valley-floor area ..... 8
Ground-water movement ..... 12
Surface water ..... 13
Streamflow ..... 13
Runoff analysis ..... 21
Magnitude and frequency of floods ..... 21
Generalized water budget ..... 23
Precipitation. ..... 26
Surface-water inflow and outflow ..... 26
Ground-water underflow ..... 26
Evapotranspiration ..... 29
Change in storage ..... 29
Water quality ..... 29
Ground water ..... 29
Surface water ..... 34
Suitability of water for irrigation ..... 34
Summary ..... 37
References. ..... 39
Supplemental information. ..... 41
System of numbering water-quality sampling sites ..... 41
System of numbering wells ..... 41
System of numbering surface-water stations ..... 41
Geologic log of test hole SC 23-72- 6AAAB1 ..... 45
Records of selected wells. ..... 48
Specific conductance and temperature of water data from selected surface-water stations ..... 54

## ILLUSTRATIONS

Plate 1. Map showing location of wells, surface-water gaging stations and potentiometric surface of the basin-fill aquifer, Wet Mountain Valley, parts of Custer and Fremont Counties, Colorado. In pocket
Figures 1-4. Maps showing:

1. Location of study area. ..... 3
2. Generalized geology ..... 4
3. Distribution of average annual precipitation, 1931-60 ..... 5
4. Availability of ground water. ..... 7
5. Diagram showing electrical resistivity section near Westcliffe. ..... 9
6. Map showing depth to water in the basin-fill aquifer. ..... 10
Page
Figure 7. Hydrographs from selected wells in the basin-fill aquifer ..... 11
7. Hydrograph of mean monthly discharge of Grape Creek near Westcliffe, 1927 to 1975 water years ..... 16
8. Hydrograph of annual runoff of Grape Creek near Westcliffe, 1925 to 1975 water years ..... 20
9. Map showing locations of water-budget tributary areas and area of basin-fill material in the Grape Creek basin ..... 28
10. Diagram for the classification of irrigation water ..... 36
11. Diagram showing U.S. Geological Survey's system of numbering water-quality sampling sites ..... 42
12. Diagram showing system of numbering wells ..... 43
TABLES
Table 1. Surface-water diversion rights prior to 1880 along Grape and Texas Creeks ..... 14
13. Principal lakes and reservoirs in the Wet Mountain Valley and vicinity ..... 17
14. Stream-gaging stations in the Wet Mountain Valley and vicinity ..... 18
15. Annual runoff in the Wet Mountain Valley ..... 22
16. Flood characteristics for selected gaging stations in the Wet Mountain Valley ..... 24
17. Generalized annual water budgets for two areas in the Wet Mountain Valley, calendar years 1931 through 1960. ..... 27
18. Chemical analyses of water from selected wells ..... 30
19. Recommended limits of selected substances in water supplies ..... 33
20. Chemical analyses of water from selected surface-water sites ..... 35
21. Salinity and sodium classification for irrigation water. ..... 38

## METRIC CONVERSION

Customary units used in this report may be expressed as metric units by use of the following conversion factors:

Multiply customary units
inch (in.)
foot ( ft )
foot per mile (ft/mi)
mile (mi)
square mile (mi ${ }^{2}$ )
acre
acre-foot (acre-ft)
gallon per minute (gal/min)
cubic foot per second ( $\mathrm{ft} \mathrm{t}^{3} / \mathrm{s}$ )
cubic foot per second per square mile $\left[\left(f t^{3} / \mathrm{s}\right) / \mathrm{mi}^{2}\right]$

By
25.40
.3048
.1894
1.609
2.590
. 4047
$1.233 \times 10^{-3}$
.06309
.02832
.01093

To obtain metric units
millimeters
meter
meter per kilometer
kilometers
square kilometers
hectare
cubic hectometers
liter per second
cubic meter per second
cubic meter per second per square kilometer

WATER-RESOURCES APPRAISAL OF THE WET MOUNTAIN VALLEY, IN PARTS OF CUSTER AND FREMONT COUNTIES, COLORADO

By Clark J. Londquist and Russell K. Livingston


#### Abstract

The Wet Mountain Valley is an intermontane trough filled to a depth of at least 6,700 feet with unconsolidated deposits. Ground water occurs under both artesian and water-table conditions within the basin-fill aquifer and groundwater movement is toward Grape and Texas Creeks. The depth to the water table is less than 10 feet in an area of about 40 square miles along the central part of the valley and is less than 100 feet in most of the remainder of the valley. Ground water stored in the upper 100 feet of saturated basin-fill sediments is estimated to total 1.5 million acre-feet.


Yields greater than 50 gallons per minute generally can be expected from wells in the central part of the basin-fill aquifer, and yields less than 50 gallons per minute are generally reported from wells around the edge of the basin-fill aquifer. Yields of wells in the mountainous areas are generally less than 10 gallons per minute.

The 540-square-mile study area is drained by two principal streams, Grape Creek and Texas Creek. Most streamflow occurs as a result of snowmelt runoff during June and July. In addition to a stream-gaging station on Grape Creek near Westcliffe, which has operated since 1924, there are two continuousrecord and four partial-record stations in the study area that were established in 1974. The long-term annual runoff at these seven stations ranges from an estimated 0.02 cubic foot per second per square mile to an estimated 1.17 cubic feet per second per square mile, generally increasing with station altitude.

DeWeese Reservoir, just outside of the study area, has a design storage capacity of 1,772 acre-feet. All other reservoirs in the area have a combined design storage capacity of about 215 acre-feet.

Generalized annual water budgets for two areas in the Wet Mountain Valley indicate that surface-water outflow is only 7 to 11 percent of the total water supply from precipitation and other sources. The remaining water is lost to the atmosphere by evapotranspiration. Ground-water underflow and changes in ground-water storage are considered insignificant in both water budgets.

The quality of both the surface and ground water is generally within the recommended limits for drinking water set by the U.S. Public Health Service. The hardness, as calcium carbonate, of the ground water is generally greater than 100 milligrams per iiter.

## INTRODUCTION

The Southeastern Colorado Water Conservancy District entered into a cooperative agreement with the U.S. Geological Survey in 1970 to conduct a water-resources investigation of the Wet Mountain Valley (fig. 1). The purpose of this investigation was to determine the quantity and quality of both the surface and the ground water within the valley. This information will be used in the management of the water resources of the area.

Westcliffe and Silver Cliff are the only towns with centralized populations in the area, each with a population of less than 400 people. The economy of the region is based primarily on agriculture. Irrigation plays a major role in crop production throughout the valley, particularly in the valley floor north and south of Westcliffe.

The scope of this investigation included the determination of the quality of water in the area, the amount of recoverable water stored in the first 100 ft of saturated alluvial material, expected yields from properly constructed wells, depths to water in the alluvial aquifer, estimates of surface-water flow from small ungaged drainage basins, and a water budget for the Grape Creek drainage area. Streamflow was measured at 7 sites, water samples for chemical analysis were collected from 20 wells and at 2 surfacewater sites, and a network of 31 wells was measured bimonthly during 1970-75 (p1. 1).

The authors wish to acknowledge the cooperation and assistance provided by the many landowners in the area, particularly those who permitted access to their wells and property for the many measurements and tests necessary for the successful completion of this investigation.

## GEOHYDROLOGIC SETTING

The Wet Mountain Valley is an intermontane trough filled with unconsolidated deposits of Cenozoic age, bordered by mountain ranges composed of volcanic rocks of Tertiary age and igneous, metamorphic, and sedimentary rocks of pre-Tertiary age (MacNish, 1966). The approximate location and extent of the various geologic units within the study area are shown on figure 2.

The study area includes about $540 \mathrm{mi}^{2}$ and is primarily drained by two north-flowing tributaries of the Arkansas River. Grape Creek, the principal stream, drains the southern $320 \mathrm{mi}^{2}$ of the study area. Texas Creek, the other large stream in the study area, drains an area of $144 \mathrm{mi}^{2}$. The remainder of the area is drained by several small streams that flow directly into the Arkansas River west of Texas Creek.

Precipitation generally increases with land-surface altitude in the Wet Mountain Valley. Based on the areal distribution of the average annual precipitation from 1931 to 1940 (fig. 3) (U.S. Weather Bureau, 1967),

$\begin{array}{llllll}0 & 5 & 10 & 15 & 20 & 25 \text { MILES }\end{array}$
$\stackrel{1}{0}$
Figure 1.-- Location of study area.


Figure 2.--Generalized geologic map.


Figure 3.-- Distribution of average annual precipitation 1931-60 (from U.S. Weather Bureau, 1967).
precipitation ranges from less than 16 in. per year on the valley floor (altitude about $8,000 \mathrm{ft}$ ) to more than 40 in . per year (altitude about $13,000 \mathrm{ft}$ ) along the Sangre de Cristo Range. About 37 percent of the total annual precipitation in the area occurs in the form of snow.

## GROUND-WATER OCCURRENCE

Ground water occurs in two different hydrologic environments within the study area--the mountainous area, which is composed chiefly of consolidated rocks, and the valley-floor area, which is underlain by unconsolidated basinfill material. The availability of ground water in the Wet Mountain Valley is summarized in figure 4.

## Mountainous Area

The Sangre de Cristo Range, on the west side of the valley, consists of igneous, metamorphic, and complexly folded and faulted sedimentary rocks of pre-Tertiary age. The Wet Mountains, on the east side of the valley, are composed of igneous and metamorphic rocks of pre-Tertiary age which are overlain by Tertiary volcanic rocks at some localities.

In the mountainous area, ground water is chiefly contained in the fractured and weathered zones of the consolidated rocks or in minor, isolated pockets of overlying glacial and alluvial material. The weathered zones of the consolidated rocks are generally limited to the upper few tens-of-feet of the rock and the fractures generally become tighter and, hence, contain less water with depth. Yields from wells completed in consolidated rock are unpredictable due to the uncertain distribution of fractures and thickness of the weathered zones but, generally, are less than 10 gal/min (fig. 4). Yields greater than $10 \mathrm{gal} / \mathrm{min}$ might be obtained where the fractures or weathered zones are in direct hydraulic connection with the water in nearby streams.

The glacial and alluvial material in the mountainous area is generally restricted to the canyons and is not widespread. Where these materials occur above the water level of nearby streams, they are generally well drained and can be expected to yield less than $10 \mathrm{gal} / \mathrm{min}$. In areas where the unconsolidated materials are in direct hydraulic connection with the water in nearby streams, well yields greater than $10 \mathrm{gal} / \mathrm{min}$ may be obtained.

In the volcanic rocks along the eastern margin of the valley, reported well yields vary considerably. In areas where the volcanic rocks are tuffaceous and of low permeability, the reported well yields are relatively small. Several wells have reportedly been drilled to depths ranging from 200 to 300 ft and have yields of less than $2 \mathrm{gal} / \mathrm{min}$. Locally, where rhyolitic rock is encountered, well yields of as much as $30 \mathrm{gal} / \mathrm{min}$ have been reported from wells of less than 50 ft deep.


Figure 4.-- Availability of ground water.

## Va11ey-F1oor Area

The basin-fill deposits in the valley-floor area cover an area of approximately $233 \mathrm{mi}^{2}$ and are the principal source of ground water in the study area. The basin-fill material is composed primarily of alluvial material derived during Tertiary time from the mountains surrounding the valley. Glaciation of the Sangre de Cristo Range has formed glacial moraine and outwash deposits which are superimposed on the Tertiary basin-fill material in some localities. Since the disappearance of the glaciers from the area, the trough has continued to slowly fill with alluvial material derived from the surrounding mountains.

An electrical resistivity survey was made from west to east across the valley about 3 mi south of Westcliffe ( 1 . 1). The resistivity section (fig. 5) indicates that the thickness of the basin-fill material ranges from about $1,000 \mathrm{ft}$ at the east end of the section to about $6,700 \mathrm{ft}$ at the west end (Zohdy and others, 1971). A test hole, SC 23-72-6AAB1 (the system of numbering wells is explained in the Supplemental Information section of this report), was drilled along the line of the section, about 0.5 mi west of Grape Creek, to a depth of $1,200 \mathrm{ft}$. The geologic 1 g of this test hole is included as Supplemental Information in this report. The log indicates that the upper $1,200 \mathrm{ft}$ of the basin-fill material, in this area, is composed primarily of sand and gravel with interbedded and intergranular clay. The resistivity section indicates that the basin-fill material contains increasing amounts of clay with depth, or that the water in the basin-fill material contains increasing amounts of dissolved solids with depth.

Ground water occurs under both artesian and water-table conditions within the basin-fill aquifer. Artesian conditions are created by interbedded and intergranular clays confining the water beneath them. These confining layers probably are not continuous or completely impermeable and there is most likely a considerable amount of leakage between the confined and unconfined parts of the aquifer. The artesian conditions exist, to some extent, throughout most of the area of the basin fill. Along the central part of the valley, there are several flowing wells (p1. 1).

The depth to the water table is less than 10 ft in an area of about $46 \mathrm{mi}^{2}$ along the central part of the valley, as shown on figure 6 . The depth to the water table is less than 100 ft in most of the remainder of the valley except for the extreme southern and southeastern parts of the valley, where it is generally greater than 100 ft .

There is an estimated 1.5 million acre-ft of recoverable ground water stored in the upper 100 ft of saturated basin-fill material. Hydrographs from selected wells are shown in figure 7. The hydrographs indicate that for the observed period of record, 1970-75, the water levels are undergoing only seasonal fluctuations and that no changes in the amount of ground water in storage are taking place.
EXPLANATION
RESISTIVITY, IN OHM-METERS



Figure 6.-- Depth to water in the basin-fill aquifer.



The basin-fill area has been divided into two areas on the basis of reported well yields (fig. 4). In the central part of the valley, wells of adequate depth, diameter, and screen length can be expected to have sustained yields of at least $50 \mathrm{gal} / \mathrm{min}$. Several wells in this area have reported yields greater than $300 \mathrm{gal} / \mathrm{min}$. (See Records of Selected Wells in the Supplemental Information section of this report.)

## GROUND-WATER MOVEMENT

Water enters the aquifers in the mountainous area as infiltration from precipitation or streams. The water then moves downgradient where it is discharged by springs to the numerous small streams, lost to evapotranspiration, or discharged into the basin-fill aquifer as underflow.

Extensive faults have been mapped along both the east and west sides of the basin between the basin-fill material and the consolidated rock (Scott and Taylor, 1975). The low permeability fault gouge associated with these faults probably serves as at least a partial barrier to water movement between the consolidated rock and the basin-fill material. This barrier and the general low permeability of the consolidated rock limit the direct discharge of water from the consolidated rock to the basin-fill material to very small amounts. Water enters the basin-fill aquifer as infiltration from precipitation falling directly on the aquifer or as rim inflow from the surrounding mountainous area.

The basin-fill material is surrounded, except for the southern end, by bedrock of low permeability. The volcanic rocks in the basin (fig. 2) are not continuous and do not serve as a conduit for transporting water out of the basin. Once the water enters the basin-fill aquifer, it moves toward the center of the basin and does not flow out of the basin to the south under Promontory Divide. Very little, if any, water leaves the basin-fill aquifer as underflow. The direction of ground-water flow in the basin-fill aquifer is shown on plate 1. Ground water moves from higher to lower potentiometric levels. As illustrated by the potentiometric map (pl. 1), ground water moves from the boundaries of the basin-fill aquifer toward Grape and Texas Creeks.

Water leaves the aquifer in several ways. Direct evapotranspiration from the water table occurs in the area where the depth to water is less than 10 ft (fig. 6). Water is discharged from the aquifer into Grape Creek along the reach from just south of Westcliffe to the edge of the basin fill and to Texas Creek in the reach just before it leaves the basin-fill material. Some water also is lost to small streams that cross the northern end of the valley-fill material and then discharge directly into the Arkansas River. Water is discharged through wells either by pumping or by artesian flow. Part of the water discharged from wells infiltrates the land surface and percolates back down to the water-table aquifer, while the remainder is consumed, lost to evapotranspiration, or runs off into the streams.

## SURFACE WATER

The surface-water resources of the Wet Mountain Valley are vital to the agricultural economy of the area. Most of the water for irrigation is obtained by diversion of surface water rather than pumping of ground water. A tabulation of water rights on Grape Creek and Texas Creek is presented in tab1e 1.

Observed streamflow consists of runoff from precipitation, return flow from irrigation, seepage from ground water, and (or) water released from reservoir storage. A hydrograph of mean monthly discharges (1925-75) of Grape Creek near Westcliffe (station 07095000; the station numbering system is explained in the Supplemental Information section of this report) is shown in figure 8. Runoff from melting snow dominates the hydrograph during May, June, and July. Streamflow during the low-flow or base-flow period in fall and winter is due largely to irrigation return flows and ground-water discharge.

The streamflow hydrograph shown in figure 8 does not indicate the effects of significant surface-water storage in the watershed. A tabulation of lakes and reservoirs in the study area and their location, design surface area, and design storage capacity is given in table 2. DeWeese Reservoir, the largest reservoir in the vicinity of the study area, is located about 0.5 mi downstream from station 07095000. The water-storage capacity of this reservoir is 1,772 acre-ft. The remaining five reservoirs in the study area have a combined water-storage capacity of about 215 acre-ft.

## Streamf1ow

The Wet Mountain Valley has large year-to-year variations in streamflow. A hydrograph of annual runoff recorded by the Grape Creek near Westcliffe gage (07095000) is shown in figure 9. From 1924 to 1975 , annual recorded runoff ranged between 5,120 acre-ft in 1963 and 78,850 acre-ft in 1942. As previously stated, the study area has only about 215 acre-ft of available water storage in surface-water reservoirs.

Other than at the Grape Creek near Westcliffe (07095000) gaging station that was established in 1924, very few records of streamflow have been collected in the study area. The streamflow-gaging station on Grape Creek near Canon City (07095500) was operated during 1907-09 and the station on Texas Creek at Texas Creek (07094000) was operated during 1923. Unpublished records of streamflow exist for Dry Creek near Westcliffe during the late 1960's and for Texas Creek at Texas Creek since September 1970.

In order to improve areal coverage of streamflow information, two continuous-record and four partial-record gaging stations were installed in August 1974 as part of this study. These stations are listed in table 3 and located on plate 1. Data obtained from these six additional stations were useful in evaluating the streamflow characteristics of the many smaller streams draining the perimeter of the study area.

Table 1.--Surface-water diversion rights prior to 1880 along Grape and Texas Creeks ${ }^{1}$

| Name of diversion | Location ${ }^{2}$ |  |  | Amount of decree, in cubic feet per second | Date of decree |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | Township | Range | Section |  |  |
| GRAPE CREEK |  |  |  |  |  |
| Aldrich No. 2- | 22 S | 072W | 32 | 0.34 | 04-15-1870 |
| Voris Bros. No. | 21S | 073W | 25 | 2.34 | 07-31-1871 |
| DeWeese Dye Main- | 195 | 070W | 06 | 2.34 | 07-31-1871 |
| Voris Bros. No. | 21S | 073W | 25 | 2.34 | 07-31-1871 |
| Pinto | 235 | 072W | 21 | . 50 | 05-01-1872 |
| Southern | 235 | 072W | 27 | 1.38 | 06-12-1873 |
| Voris Bros. No. | 215 | 072W | 25 | . 95 | 10-31-1873 |
| DeWeese Dye Main | 195 | 070W | 06 | . 95 | 10-31-1873 |
| Voris Bros. No. 1 | 215 | 072W | 25 | . 95 | 10-31-1873 |
| Difz | 235 | 072W | 27 | . 83 | 06-15-1874 |
| Rifster No. | 235 | 072W | 34 | . 88 | 10-10-1874 |
| C B H | 235 | 072W | 34 | . 50 | 10-10-1874 |
| Pinto | 235 | 072W | 21 | 2.50 | 03-03-1875 |
| Gordon W. Smith No. | 225 | 072W | 19 | 3.00 | 05-15-1875 |
| Elze No. 3 | 24 S | 072W | 04 | . 69 | 05-31-1875 |
| Elze No. 5- | 245 | 072W | 03 | . 16 | 05-31-1875 |
| Voris Jarvis No. | 22S | 073W | 01 | 2.61 | 05-31-1875 |
| Risser and Lock | 23 S | 072W | 05 | 5.11 | 06-01-1875 |
| Hulmuth | 23S | 072W | 16 | 2.50 | 10-05-1875 |
| R B- | 23 S | 072W | 20 | . 89 | 05-05-1876 |
| Chetelate No. | 23S | 072W | 20 | . 33 | 06-01-1876 |
| Schulz No. 3 | 24 S | 072W | 03 | . 66 | 06-01-1876 |
| C B | 46 N | 012 E | 13 | 1.30 | 06-05-1876 |
| Colfax | 23S | 072W | 04 | 2.33 | 06-10-1876 |
| Voris Bros. No. 2 | 22 S | 073W | 01 | 1.33 | 06-30-1877 |
| DeWeese Dye Main | 195 | 070W | 06 | 1.33 | 06-30-1877 |
| Voris Bros. No. 2 | 22S | 073W | 01 | 1.33 | 06-30-1877 |
| Schoolfield No. | 22S | 072W | 05 | . 56 | 04-01-1878 |
| Schulz No. 5- | 245 | 072W | 03 | . 33 | 05-15-1878 |
| Voris Bros. No. 1, 2d | 215 | 073W | 25 | 1.09 | 10-31-1879 |
| DeWeese Dye Main | 195 | 070W | 06 | 1.09 | 10-31-1879 |
| Voris Bros. No. 1, 2d | 215 | 073W | 25 | 1.09 | 10-31-1879 |

[^0]Table 1.--Surface-water diversion rights prior to 1880 along Grape and Texas Creeks ${ }^{1}$--Continued

| Name of diversion | Location ${ }^{2}$ |  |  | Amount of decree, in cubic feet per second | Date of decree |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | Township | Range | Section |  |  |
| TEXAS CREEK |  |  |  |  |  |
| Hill No. | 47N | 012E | 12 | 1.05 | 08-31-1869 |
| Hill No. 2 | 47N | 012E | 12 | 1.05 | 08-31-1869 |
| Pitter No. 1 | 215 | 073W | 29 | 1.00 | 04-30-1871 |
| Likely \& McCormick | 47N | 012 E | 14 | 1.32 | 06-01-1871 |
| McCormick | 47N | 012 E | 13 | 1.00 | 06-01-1871 |
| Mill | --- | ---- | -- | 1.00 | 09-01-1871 |
| Ritter No. 2- | 215 | 073W | 29 | 1.00 | 05-01-1872 |
| Duckett No. | 47N | 012 E | 26 | 1.00 | 05-10-1872 |
| Belknap \& Hendrickson | 47N | 012 E | 35 | 1.00 | 05-10-1872 |
| Duckett No. 1- | 47N | 012 E | 26 | 1.00 | 05-10-1872 |
| Belknap \& Howard | 46N | 012 E | -- | 2.76 | 05-31-1872 |
| Dissmore No. 2 McClurk | 215 | 073W | 31 | 1.00 | 07-24-1872 |
| McClurken No. 1- | 215 | 073W | 31 | 1.00 | 07-30-1872 |
| Hugg No. 3 | 215 | 073W | 32 | 1.06 | 11-30-1872 |
| Hugg No. 4 | 215 | 073W | 32 | 1.00 | 11-30-1872 |
| Duckett No. 2 | 47N | 012 E | 26 | 1.32 | 05-15-1873 |
| Hill Ditch No. 2 | 47N | 012 E | 12 | 1.05 | 01-17-1874 |
| Myers | 215 | 073W | 16 | 1.00 | 05-01-1874 |
| J. M. Duckett | 47N | 012 E | 34 | 3.50 | 05-31-1874 |
| Burgman No. 2 | 47N | 012 E | 33 | 1.00 | 04-30-1875 |
| Hill No. | 47N | 012 E | 12 | 1.05 | 04-14-1876 |
| Likely | 47N | 012 E | 12 | 1.60 | 03-06-1877 |
| Belknap \& Hendrickson | 47N | 012 E | 35 | 1.58 | 05-31-1878 |

Total------------------------------------------ 29.34
${ }^{1}$ Source: Colorado Division of Water Resources, Office of the State Engineer.
${ }^{2}$ Refers to location of point of diversion.


Table 2.--Principal lakes and reservoirs in the Wet Mountain Valley and vicinityl

| Name | Location ${ }^{2}$ |  |  | Owner | ```Design surface area, in acres``` | Design capacity, in acre-feet |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Township | Range | Section |  |  |  |
| Balman Reservoir-- | 46 N | 12 E | 6 | U.S. Forest Service | 3.6 | 50.9 |
| Hook Reservoir---- | 46N | 12E | 2 | Thomas Hook | 2.0 | 35.0 |
| DeWeese Reservoir- | 215 | 72W | 20 | DeWeese Dye Ditch and Reservoir Co. | 148 | 1,772 |
| Lake of the Clouds Reservoir No. 1- | 45 N | 12E | (21) | U.S. Forest Service | 15.0 | 48.0 |
| Lake of the Clouds Reservoir No. 2- | 45 N | 12E | (21) | -do- | 15.0 | 36.0 |
| Lake of the Clouds Reservoir No. 3- |  | 12 E | (21) | ---do-- | (15) | (45) |
| ${ }^{1}$ Source: Col Engineer. <br> ${ }^{2}$ Refers to 10 | orado <br> cation | Divisi <br> of dam | on of or out | Water Resources, 0 let. | fice of | the State |

Table 3.--Stream-gaging stations in

| Stream site number ${ }^{1}$ | U.S. Geological Survey station number | Station name | Operating agency | Location |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Latitude | Longitude |
| 1 | 07093980 | Lake Creek near Hillside, Colo- | U.S. Geological Survey | $38^{\circ} 15^{\prime} 49^{\prime \prime}$ | $105^{\circ} 39^{\prime} 41^{\prime \prime}$ |
| 2 | 07094000 | Texas Creek at Texas Creek (near Cotapaxi), Colo---- | Office of the Colorado State Engineer | $38^{\circ} 24^{\prime} 30^{\prime \prime}$ | $105^{\circ} 35^{\prime} 00^{\prime \prime}$ |
| 3 | 07094530 | Grape Creek near Bradford, Colo- | U.S. Geological Survey | $37^{\circ} 57^{\prime} 03^{\prime \prime}$ | $105^{\circ} 27^{\prime} 03^{\prime \prime}$ |
| 4 | 07094600 | South Colony Creek near Westcliffe, Colo------------ | --do---------- | $37^{\circ} 59^{\prime \prime} 57^{\prime \prime}$ | $105^{\circ} 29^{\prime} 25^{\prime \prime}$ |
| 5 | 07094700 | Antelope Creek near Rosita, Colo----------- | -do--------- | $38^{\circ} 01^{\prime \prime} 18^{\prime \prime}$ | $105^{\circ} 21^{\prime \prime} 43^{\prime \prime}$ |
| 6 | 07094800 | Venable Creek near Westcliffe, Colo--- | --do--------- | $38^{\circ} 05^{\prime} 02^{\prime \prime}$ | $105^{\circ} 33^{\prime} 52^{\prime \prime}$ |
| 7 | 07094900 | Middle Taylor Creek near Westcliffe, Colo----------- | --do---------- | $38^{\circ} 06^{\prime} 30^{\prime \prime}$ | $105^{\circ} 36^{\prime} 03^{\prime \prime}$ |
| 8 | 07095000 | Grape Creek near Westcliffe, Colo----------- | Office of the Colorado State Engineer | $38^{\circ} 11^{\prime} 10^{\prime \prime}$ | $105^{\circ} 28^{\prime} 59^{\prime \prime}$ |

[^1]the Wet Mountain Valley and vicinity

| $\begin{gathered} \text { Drainage } \\ \text { area } \\ \left(m i^{2}\right) \end{gathered}$ | Period of record ${ }^{2}$ | Mean annual discharge ( $\mathrm{ft}^{3} / \mathrm{s}$ ) | Maximum observed discharge ( $\mathrm{ft}^{3} / \mathrm{s}$ ) | Minimum observed discharge ( $\mathrm{ft}^{3} / \mathrm{s}$ ) | Remarks |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 9.07 | $\begin{aligned} & \text { June } 1974- \\ & \text { September } 1976 \end{aligned}$ | ----- | 20 | 1.6 | Partial-record station, discharge measurements only. |
| 144 | April-November 1923 | ----- | 2,800 | ---- | Records only for 1923 are published. |
| 6.72 | $\begin{aligned} & \text { June } 1974- \\ & \text { September } 1976 \end{aligned}$ | ----- | 19 | . 3 | Partial-record station, discharge measurements only. |
| 6.03 | October 1974September 1976 | 6.65 | 47 | . 20 |  |
| 35.3 | $\begin{aligned} & \text { June } 1974- \\ & \text { September } 1976 \end{aligned}$ | ----- | 9.4 | 0 | Do. |
| 2.55 | $\begin{aligned} & \text { June } 1974- \\ & \text { September } 1976 \end{aligned}$ | ----- | 17 | . 04 | Do. |
| 3.19 | October 1974September 1976 | 4.10 | 38 | . 40 |  |
| 320 | October 1924- <br> September 1961; <br> October 1962- <br> September 1976 | 32.3 | 7,460 | . 01 |  |

determined; records may exist for other periods.

Figure 9.-- Annual runoff of Grape Creek near Westcliffe (station number 07095000), 1925 to 1975 water years.

## Runoff Analysis

The results of an analysis of runoff in the study area are summarized in table 4. The 1975-76 annual runoff for the four partial-record stations was determined by correlation techniques using the two continuous-record stations and is expected to be within 10 percent of the actual runoff (Riggs, 1969). Observed annual runoff in the study area was generally greatest during the 1975 water year.

Long-term annual runoff (1931-60), also given in table 4, was estimated for the six short-term gaging stations based on the 1975-76 average annual runoff, channel-geometry techniques (Hedman and others, 1972), and regression analysis of basin characteristics (Livingston, 1970). Equations for predicting the long-term mean annual runoff using channel-geometry measurements have a standard error of estimate of 19 percent; those for basin characteristics, 47 percent. (The standard error of estimate is the percentage above or below calculated values within which about two-thirds of the actual values are expected to fall.) The recorded 1975-76 average annual runoff for Grape Creek near Westcliffe is in close agreement with the long-term (1931-60) runoff. Consequently, the 1975-76 average annual runoff for the six short-term gaging stations, along with the determinations made using channel-geometry and regression methods, were averaged to estimate the long-term (1931-60) mean annual runoff at these stations. These data were used in the water-budget analysis which is discussed in a later section of this report.

Annual runoff expressed as a function of drainage area, in cubic feet per second per square mile, generally increases with altitude in the Wet Mountain Valley, as shown in table 4. Values of estimated mean annual runoff for 1931 through 1960 range from about $0.02\left(\mathrm{ft}^{3} / \mathrm{s}\right) / \mathrm{mi}^{2}$ for Antelope Creek (altitude $8,290 \mathrm{ft}$ ) to about $\left.1.17 \mathrm{ft}^{3} / \mathrm{s}\right) / \mathrm{mi}^{2}$ for Middle Taylor Creek(altitude $9,960 \mathrm{ft}$ ). Such data can be used to give an estimate of annual runoff at ungaged sites in the study area. For example, the data shown in table 4 indicate that the annual runoff for a stream with a $5-\mathrm{mi}^{2}$ drainage area at an altitude of $9,000 \mathrm{ft}$ is about $5 \mathrm{ft}^{3} / \mathrm{s}$ or $1.0\left(\mathrm{ft}^{3} / \mathrm{s}\right) / \mathrm{mi}^{2}$.

## Magnitude and Frequency of Floods

Occasional flooding in the Wet Mountain Valley intensive rainstoms or unusual snowmelt-runoff conditions. magnitude and frequency of floods can be used by planners minimize damages and hazards in areas subject to flooding.

Where adequate records of flood discharge are available, a relation can be developed between flood magnitude and frequency of occurrence. The logPearson Type III distribution has been established as the method for uniform flood-frequency analyses by all Federal agencies (U.S. Water Resources Council, 1976). In order to reduce the effects of possible time-sampling errors associated with such analyses, a weighting procedure can be used (Sauer, 1974; McCain and Jarrett, 1976). For example, the 100-year flood discharge for Grape Creek at Westcliffe (07095000) is $4,900 \mathrm{ft}^{3} / \mathrm{s}$ by log-

| Station name (station number) | Drainage area, in square miles | Alti- <br> tude, in feet | Observed annual runoff, in acre-feet |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | $1975$ <br> Water year | 1976 <br> Water year | $1975-76$ <br> Average |
| Lake Creek near Hillside (07093980)4_- | $9.07$ | 8,300 | 3,590 | 3,610 | 3,600 |
| Grape Creek near Bradford (07094530) ${ }^{4}$ | 6.72 | 8,690 | 3,330 | 2,960 | 3,140 |
| South Colony Creek near Westcliffe (07094600) | $6.03$ | 8,940 | 5,060 | 4,570 | 4,820 |
| Antelope Creek near <br> Rosita (07094700)4--- | $35.3$ | 8,290 | 780 | 50 | 420 |
| Venable Creek near Westcliffe (07094800) ${ }^{4}$ | $2.55$ | 9,020 | 2,160 | 1,510 | 1,840 |
| Middle Taylor Creek near Westcliffe (07094900) | $3.19$ | 9,960 | 3,700 | 2,240 | 2,970 |
| Grape Creek near Westcliffe (07095000) | $-320$ | 7,790 | 33,710 | 14,780 | 24,240 |
| ${ }^{1}$ Hedman, Moore, and <br> ${ }^{2}$ Livingston, 1970. <br> ${ }^{3}$ Figures in parent regression methods (see <br> ${ }^{4}$ Partial-record sta <br> ${ }^{5}$ Value not used in | Livings <br> theses w text). tion. determin | $\text { on, } 197$ <br> e det <br> ng mean | rmined as <br> annual runof | mean of th | observed |

Pearson Type III analysis and $9,370 \mathrm{ft}^{3} / \mathrm{s}$ by the weighting procedure. A complete summary of the flood characteristics for Grape Creek at Westcliffe (07095000), including the equations and parameter values used in their determination, is given in table 5.

For natural-flow streams where adequate records of flood discharge are not available, the flood characteristics can be estimated using regional relations given in a manual developed by McCain and Jarrett (1976). The method presented in the manual involves the use of basin characteristics and regionalized regression equations. A summary showing the use of this method to determine the flood characteristics at two partial-record stations, Antelope Creek near Rosita (07094700) and Venable Creek near Westcliffe (07094500) is given in table 5.

Wet Mountain Valley

| Estimated annual runoff, in acre-feet |  | Mean annual runoff (1931-60 calendar years) ${ }^{3}$ |  |
| :---: | :---: | :---: | :---: |
| Channel-geometry method ${ }^{1}$ | Regression method ${ }^{2}$ | Acre-feet | Cubic feet per second per square mile |
| 3,560 | 55,170 | $(3,600)$ | (0.55) |
| 51,120 | 2,880 | $(3,000)$ | (.62) |
| 2,920 | 5,930 | $(4,500)$ | (1.03) |
| 380 | ----- | (400) | (.02) |
| 1,830 | 1,930 | $(1,900)$ | (1.03) |
| 2,260 | 2,850 | $(2,700)$ | (1.17) |
| ----- | ----- | 22,460 | . 10 |

1975-76 average runoff and the estimated runoff by channel-geometry and

GENERALIZED WATER BUDGET
The movement of water in a particular area may be expressed as a water budget in which an accounting is made of all water entering or leaving the area. The water budget can be expressed in equation form as:

Water in $=$ Water out $\pm$ change in storage,
where "water in" includes precipitation and surface- and ground-water inflow, "water out" includes evapotranspiration and surface- and ground-water outflow, and "change in storage" includes both surface- and ground-water storage and is positive for a net increase in storage.

# Table 5.--Flood characteristics for selected 

Station name and number

Basin type Required parameters

Antelope Creek
near Rosita (07094700)----- Ungaged site-mixed population (plains/ mountain) region
$Q_{T}=d i s c h a r g e ~ f o r ~ r e c u r r e n c e ~ i n t e r v a l, ~ T, ~$ $D_{T}^{\top}=$ depth for recurrence interval, $T$, $A=35.3 \mathrm{mi}^{2}$ (drainage area) ( $9.45 \mathrm{mi}^{2}$, plains region; $25.8 \mathrm{mi}^{2}$, mountain region), $\mathrm{P}=18 \mathrm{in}$.
=mean annual precipitation
(mountain region only),
$S_{B}=114 \mathrm{ft} / \mathrm{mi}$ (basin slope)
$\mathrm{S}_{\mathrm{S}}=64 \mathrm{ft} / \mathrm{mi}$ (streambed slope)
$R=0.70$ (reduction factor for site elevation).

Venable Creek near Westcliffe (07094900)--.- Ungaged site-- $\quad A=2.55 \mathrm{mi}^{2}$ (drainage area), mountain region $\mathrm{P}=28 \mathrm{in}$. (mean annual precipitation).

Grape Creek near Westcliffe (07095000)-----

Gaged site--mixed population (plains/mountain) region
$\mathrm{Q}_{\mathrm{T}(\mathrm{W})}=$ weighted discharge for recur-
$Q_{T(S)}=$ station value of the flood for recurrence interval, $\mathrm{T}^{1}$,
$Q_{T(R)}=$ regression value of the flood $N=46$ (number of years of station
data used to compute $\mathrm{Q}_{\mathrm{T}(\mathrm{S})}$ ), $E=10$ (equivalent years of record for $Q_{T(R)}$ ), $A=320 \mathrm{mi}^{2}$ (drainage area)
(220 mi ${ }^{2}$, plains region; $100 \mathrm{mi}^{2}$, mountain region), $\mathrm{P}=19$ in., $\mathrm{S}_{\mathrm{B}}=47 \mathrm{ft} / \mathrm{mi}$, $\mathrm{S}_{\mathrm{S}}=29 \mathrm{ft} / \mathrm{mi}$.
${ }^{1}$ Method of determination as described by McCain and Jarrett, 1976.
${ }^{2}$ Standard error of estimate is the percentage above or below calculated ${ }^{3}$ Highest flood characteristic obtained by the two methods used; in this

| Regression <br> equation | Standard error <br> of estimate <br> (percent) |
| :---: | :---: | Resultant flood characteristic

Plains region:

$$
\begin{array}{ll}
\mathrm{Q}_{10}=144 \mathrm{~A}^{0.528} \mathrm{~S}_{\mathrm{B}}^{0.336} & 31 \\
\mathrm{Q}_{50}=891 \mathrm{~A}^{0.482} \mathrm{~S}_{\mathrm{B}}^{0.154} & 24 \\
\mathrm{Q}_{100}=1,770 \mathrm{~A}^{0.463} \mathrm{~S}_{\mathrm{B}}^{0.086} & 28 \\
\mathrm{Q}_{500}=5,770 \mathrm{~A}^{0.432} & 45 \\
\mathrm{D}_{100}=59.3 \mathrm{~S}_{\mathrm{S}}^{-0.517} & 21
\end{array}
$$

Mountain region:

$$
\mathrm{Q}_{10}=0.12 \mathrm{~A}^{0.815} \mathrm{pl} .592
$$

$$
\mathrm{Q}_{50}=0.91 \mathrm{~A}^{0.795 \mathrm{p} 1.110} 37
$$

$$
Q_{100}=1.88 \mathrm{~A}^{0.787} \mathrm{p} 0.932
$$

$$
38
$$

$$
\mathrm{Q}_{500}=8.70 \mathrm{~A}^{0.766} \mathrm{p}^{0.560}
$$

$$
\mathrm{D}_{100}=1.44 \mathrm{~A}^{0.187} \mathrm{P}^{0.059}
$$28

(Equations given above)

$$
Q_{T(W)}=\frac{\left(Q_{T}(S)^{\times N)+\left(Q_{T}(R)^{\times E}\right)}\right.}{N+E}
$$

$$
\begin{aligned}
& \mathrm{Q}_{10}=52 \mathrm{ft}^{3} / \mathrm{s} \\
& \mathrm{Q}_{50}=77 \mathrm{ft}^{3} / \mathrm{s} \\
& \mathrm{Q}_{100}=88 \mathrm{ft}^{3} / \mathrm{s} \\
& \mathrm{Q}_{500}=115 \mathrm{ft}^{3} / \mathrm{s} \\
& \mathrm{D}_{100}=2.1 \mathrm{ft} \\
& \\
& { }^{3} \mathrm{Q}_{10}(\mathrm{~W})=2,780 \mathrm{ft}^{3} / \mathrm{s} \\
& { }^{3} \mathrm{Q}_{50}(\mathrm{~W})=6,740 \mathrm{ft}^{3} / \mathrm{s} \\
& { }^{3} \mathrm{Q}_{100}(\mathrm{~W})=9,370 \mathrm{ft}^{3} / \mathrm{s} \\
& { }^{3} \mathrm{Q}_{500}(\mathrm{~W})=18,800 \mathrm{ft} 3 / \mathrm{s} \\
& { }^{3} \mathrm{D}_{100}(\mathrm{~W})=10.4 \mathrm{ft}
\end{aligned}
$$

(Equations for $Q_{T(R)}$ and $\mathrm{D}_{100}$ given above)
values within which about two-thirds of the actual values are expected to fall. instance, the characteristics from the plains-region determination are used.

Generalized annual water budgets are given in table 6 for the Grape Creek drainage basin above the Grape Creek near Westcliffe gaging station (07095000), and for that part of this drainage basin consisting of basin-fill material (fig. 10). The area of basin-fill drained by Grape Creek represents $167 \mathrm{mi}^{2}$ of the $320-\mathrm{mi}^{2}$ drainage basin. The budgets are based on calendar years 1931 through 1960.

## Precipitation

The average annual precipitation falling on the water-budget areas was determined by use of the distribution of mean annual precipitation for 1931-60 shown on figure 3. The isohyetal method of analysis resulted in a 331,000-acre-ft average for the entire drainage area and a 151,000-acre-ft average for the basin-fill area.

## Surface-Water Inflow and Outflow

There is no surface-water inflow across the Grape Creek drainage divide, such as transmountain diversions. Surface-water inflow to the basin-fill area consists of runoff from the mountainous regions along the perimeter of the area. For purposes of the water budget, the $153 \mathrm{mi}^{2}$ that contribute inflow were separated into the two tributary areas shown on figure 10 . Tributary area $A\left(72 \mathrm{mi}^{2}\right)$, which extends from the Sangre de Cristo Range divide to the basin-fill boundary, receives the most precipitation within the study area (fig. 3) and is consequently heavily vegetated. Tributary area B ( $81 \mathrm{mi}^{2}$ ), on the other hand, generally extends from the Wet Mountain divide to the basinfill boundary and is characteristically dry and sparsely vegetated except at higher altitudes. Runoff from the two tributary areas was determined using the estimated 1931-60 mean annual runoff for the six gaging stations (table 4), with consideration given to the relative characteristics of these basins and the ungaged area. Runoff data from several other hydrologically similar gaged basins outside the Wet Mountain Valley also were considered. The average annual yield during 1931-60 was estimated to be 50,000 acre-ft from tributary area $A$ and 4,000 acre-ft from tributary area B.

Surface-water outflow from both the entire drainage basin and the basinfill part of the basin was measured by the Grape Creek near Westcliffe gaging station (07095000). During the 1931 through 1960 calendar years, the flow past this gage averaged about 22,000 acre-ft annually.

## Ground-Water Underflow

Ground-water underflows into or out of both water-budget areas are assumed negligible because low-permeable pre-Tertiary rocks are located along most of the perimeter of the basin. The potentiometric map (pl. 1) indicates that ground-water does not flow across the Grape Creek-Texas Creek drainage divide or the Promontory Divide (Custer-Huerfano County line). Consequently, there are no significant ground-water underflows out of the water-budget areas.

| Grape Creek budget area | Water in |  |  |  | Water out |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Source | Acre-feet | Inches | Percenttage | Source | Acre-feet | Inches | $\begin{gathered} \text { Percent- } \\ \text { age } \end{gathered}$ |
| Entire drainage areal${ }^{1}--------$ | Precipitation ${ }^{2}$ Surface water----Ground-water underflow ${ }^{4}$----- | $331,000$ $0$ | 19.4 0 0 | $\begin{array}{r} 100 \\ 0 \\ 0 \end{array}$ | Evapotranspiration ${ }^{3}$-- <br> Surface water²------- <br> Ground-water <br> underflow ${ }^{4}--------$ <br> Change in storage ${ }^{4}$--- | $\begin{array}{r} 309,000 \\ 22,000 \\ 0 \\ 0 \end{array}$ | $\begin{gathered} 18.1 \\ 1.3 \\ 0 \\ 0 \end{gathered}$ | $\begin{array}{r} 93 \\ 7 \\ 0 \\ 0 \end{array}$ |
|  | Totals | 331,000 | 19.4 | 100 | Totals | 331,000 | 19.4 | 100 |
| $\begin{gathered} \text { Basin-fill } \\ \text { area }{ }^{5}-- \end{gathered}$ | Precipitation²--- <br> Surface water (tributary area A) ${ }^{6}-------$ <br> Surface-water (tributary area B) ${ }^{6}$------- <br> Ground-water underflow ${ }^{4}$---- | $\begin{array}{r} 151,000 \\ 50,000 \\ 4,000 \\ 0 \end{array}$ | $\begin{aligned} & 17.0 \\ & 5.6 \\ & 0.4 \\ & 0 \end{aligned}$ | $74$ <br> 24 <br> 2 <br> 0 | Evapotranspiration ${ }^{3}--$ <br> Surface water ${ }^{2}$ <br> Ground-water underflow ${ }^{4}$ <br> Change in storage ${ }^{4}-$-- | $183,000$ $22,000$ <br> 0 <br> 0 | 20.5 2.5 0 0 | 89 11 0 0 |
|  | Totals | 205,000 | 23.0 | 100 | Totals | 205,000 | 23.0 | 100 |
| ${ }^{1}$ Drainage basin above Grape Creek near Westcliffe (07095000), location shown on figure $320 \mathrm{mi}^{2}$. <br> ${ }^{2}$ Based on recorded data. <br> ${ }^{3}$ Estimated as all water otherwise unaccounted for in the budget. <br> ${ }^{4}$ Assumed negligible. <br> ${ }^{5}$ That part of the Grape Creek near Westcliffe (07095000) drainage composed of bas figure 11). This is an area of $167 \mathrm{mi}^{2}$. <br> ${ }^{6}$ Estimated (see text). |  |  |  |  |  |  |  |  |

EXPLANATION
AREA OF BASIN-FILL MATERIAL
TRIBUTARY AREA A
TRIBUTARY AREA A
TRIBUTARY AREA B
AREA BOUNDARY



10 MILES
<
7

$\rangle$

1.

L
$\stackrel{O}{1}$

The evapotranspiration component of the water budgets was estimated to be all water unaccounted for in the budget. The result of this determination is 309,000 acre-ft or 18.1 in. for the entire drainage basin, and 183,000 acreft or 20.5 in . for the basin-fill area. These estimates are in close agreement with the results of several detailed studies conducted elsewhere in Colorado. Wymore (1974) estimated that the average annual evapotranspiration from the $142-\mathrm{mi}^{2}$ upper Piceance Creek basin is 18.47 in. Kruse and Haise (1974) measured 23.2 to 24.5 in. of evpotranspiration from high mountain meadows in the South Park area, and 23.5 to 27.8 in. in the Gunnison area.

## Change in Storage

Because there are no large surface-water reservoirs in the water-budget area and ground-water levels measured in wells during the 24 -month period were relatively stable, this component of the budgets was assumed to be zero.

## WATER QUALITY

## Ground Water

Water samples were collected and analyzed from 20 wells within the basin (pl. 1). The results of these analyses are shown in table 7. (The system of numbering water-quality sampling sites is explained in the Supplemental Information section of this report.) The analyses indicate that, except for four wells discussed below, the constituents found in ground water in the basin are within the recommended limits for drinking water (table 8) set by the U.S. Public Health Service (1962) and the U.S. Environmental Protection Agency (1972).

The concentrations of dissolved solids, $703 \mathrm{mg} / \mathrm{L}$ (milligrams per liter), and dissolved nitrite plus nitrate, $16 \mathrm{mg} / \mathrm{L}$, exceeded the recommended limits of $10 \mathrm{mg} / \mathrm{L}$ in water from well SC $22-72-31 \mathrm{ADB}$. As this well is a $15-\mathrm{ft}$ deep stock well, located in a feed lot, the water chemistry is not typical of the water from the major part of the valley. Dissolved solids greater than $500 \mathrm{mg} / \mathrm{L}$ may impart a bitter taste to the water. Dissolved nitrite plus nitrate greater than $10 \mathrm{mg} / \mathrm{L}$ is generally indicative of pollution of the water from animal wastes and (or) fertilizers and may cause methemoglobinemia (bluebaby disease) in infants (less than 6 months of age) who drink the water or who are breast-fed by mothers who drink the water.

The main significance of the above sampled well shows the relative ease with which surface and near-surface sources may pollute a normally potable domestic water supply. Therefore, it is important that proper consideration be given in the future to waste-disposal practices and their consequent effects upon the ground-water supply, especially in the $46-\mathrm{mi}^{2}$ area in which the depth to water is less than 10 ft .
Table 7.--Chemical analyses of water from selected wells



| Site number ${ }^{1}$ | Dissolved magnesium (Mg) (mg/L) | Dissolved sodium ( Na ) (mg/L) | Dissolved potassium (K) (mg/L) | Bicarbonate $\left(\mathrm{HCO}_{3}\right)$ (mg/L) | Carbonate $\left(\mathrm{CO}_{3}\right)$ (mg/L) | Alkalinity as $\mathrm{CaCO}_{3}$ (mg/L) | Dissolved sulfate $\left(\mathrm{SO}_{4}\right)$ (mg/L) | Dissolved chloride (C1) (mg/L) | Dissolved fluoride (F) ( $\mathrm{mg} / \mathrm{L}$ ) | Dis- solved nitrite plus nitrate $(\mathrm{N})$ $(\mathrm{mg} / \mathrm{L})$ | Dissolved ortho-phosphorus (P) $(\mathrm{mg} / \mathrm{L})$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 14 | 21 | 2.1 | 263 | 0 | 220 | 82 | 7.1 | 0.5 | 0.51 | 0.02 |
| 10 | 7.2 | 7.5 | 1.4 | 151 | 0 | 124 | 9.5 | 2.3 | . 2 | . 52 | . 01 |
| 16 | 7.9 | 5.6 | 1.1 | 113 | 0 | 93 | 22 | 1.0 | . 2 | . 43 | . 01 |
| 19 | 10 | 8.0 | . 9 | 149 | 0 | 122 | 21 | . 9 | . 2 | . 16 | . 01 |
| 22 | 17 | 12 | 2.8 | 192 | 0 | 157 | 48 | 25 | . 6 | 3.6 | . 02 |
| 26 | 8.6 | 17 | 2.6 | 183 | 0 | 150 | 21 | 8.2 | . 6 | 1.1 | . 03 |
| 32 | 14 | 39 | 200 | 359 | 0 | 294 | 63 | 69 | . 5 | 16 | . 55 |
| 34 | 13 | 16 | 4.8 | 333 | 0 | 273 | 13 | 6.8 | . 4 | . 00 | . 01 |
| 37 | 5.2 | 7.9 | . 4 | 136 | 0 | 112 | 8.5 | . 9 | . 2 | . 12 | . 00 |
| 46 | 7.3 | 9.4 | 1.4 | 145 | 0 | 119 | 11 | 1.2 | . 2 | . 11 | . 01 |
| 66 | 4.7 | 3.8 | 1.0 | 85 | 0 | 70 | 8.5 | . 9 | . 1 | . 06 | . 00 |
| 74 | 2.5 | 15 | 1.1 | 125 | 0 | 103 | 4.3 | 1.6 | . 2 | . 09 | . 01 |
| 101 | 8.6 | 9.0 | 1.4 | 198 | 0 | 163 | 7.2 | 1.2 | . 2 | . 75 | . 02 |
| 108 | 8.3 | 12 | 3.7 | 225 | 0 | 185 | 12 | 3.6 | . 3 | . 38 | . 02 |
| 112 | 6.5 | 13 | 3.5 | 191 | 0 | 157 | 11 | 3.8 | . 6 | . 29 | . 07 |
| 114 | 4.9 | 6.5 | 1.2 | 123 | 0 | 101 | 7.4 | 3.3 | . 3 | . 42 | . 03 |
| 118 | 5.8 | 4.7 | . 9 | 170 | 0 | 139 | 4.7 | . 9 | . 1 | . 07 | . 02 |
| 127 | 12 | 14 | 6.1 | 261 | 0 | 214 | 35 | 31 | . 3 | 14 | . 35 |
| 144 | 17 | 14 | 2.3 | 226 | 0 | 185 | 23 | 2.0 | 1.8 | . 01 | . 01 |
| 153 | 2.9 | 4.6 | . 7 | 106 | 0 | 87 | 2.6 | . 9 | . 1 | . 02 | . 01 |



| Site number ${ }^{1}$ | Dissolved ortho-phosphate ( $\mathrm{PO}_{4}$ ) (mg/L) | Dis- solved solids (sum of constit- uents) (mg/L) | Dissolved solids (tons per acre-ft) | $\begin{aligned} & \text { Hard- } \\ & \text { ness } \\ & \text { as } \\ & \mathrm{CaCO}_{3} \\ & (\mathrm{Ca}, \mathrm{Mg}) \\ & (\mathrm{mg} / \mathrm{L}) \end{aligned}$ | Non- <br> carbonate hardness (mg/L) | Percent sodium | Sodium-ad-sorption ratio (SAR) | Specific conductance ( $\mu \mathrm{mhos} / \mathrm{cm}$ at $25^{\circ} \mathrm{C}$ ) | pH | Tem-pera- <br> ture <br> (de- <br> grees <br> Cel- <br> sius) | Carbon dioxide $\left(\mathrm{CO}_{2}\right)$ (mg/L) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 0.06 | 367 | 0.50 | 260 | 43 | 15 | 0.6 | 590 | 7.4 | 15.0 | 17 |
| 10 | . 03 | 157 | . 21 | 110 | 0 | 12 | . 3 | 262 | 7.5 | 9.5 | 7.6 |
| 16 | . 03 | 139 | . 19 | 100 | 7 | 11 | . 2 | 226 | 7.7 | 10.5 | 3.6 |
| 19 | . 03 | 169 | . 23 | 130 | 4 | 12 | . 3 | 273 | 7.5 | 12.5 | 7.5 |
| 22 | . 06 | 305 | . 41 | 220 | 62 | 10 | . 4 | 493 | 7.6 | 15.5 | 7.7 |
| 26 | . 09 | 225 | . 31 | 140 | 0 | 20 | . 6 | 357 | 7.6 | 15.0 | 7.4 |
| 32 | 1.7 | 703 | . 96 | 170 | 0 | 16 | 1.3 | 1,110 | 7.5 | 12.0 | 18 |
| 34 | . 03 | 328 | . 45 | 250 | 0 | 12 | . 4 | 543 | 7.4 | 10.0 | 21 |
| 37 | . 00 | 138 | . 19 | 100 | 0 | 14 | . 3 | 230 | 7.2 | 9.0 | 14 |
| 46 | . 03 | 155 | . 21 | 110 | 0 | 16 | . 4 | 247 | 7.6 | 11.5 | 5.8 |
| 66 | . 00 | 94 | . 13 | 74 | 5 | 10 | . 2 | 154 | 7.4 | 10.0 | 5.4 |
| 74 | . 03 | 138 | . 19 | 75 | 0 | 30 | . 8 | 206 | 7.7 | 17.0 | 4.0 |
| 101 | . 06 | 188 | . 26 | 150 | 0 | 11 | . 3 | 324 | 7.6 | 10.0 | 8.0 |
| 108 | . 06 | 238 | . 32 | 170 | 0 | 13 | . 4 | 382 | 7.7 | 9.5 | 7.2 |
| 112 | . 21 | 207 | . 28 | 140 | 0 | 17 | . 5 | 329 | 7.6 | 15.0 | 7.7 |
| 114 | . 09 | 138 | . 19 | 95 | 0 | 13 | . 3 | 217 | 7.9 | 10.5 | 2.5 |
| 118 | . 06 | 157 | . 21 | 130 | 0 | 7 | . 2 | 266 | 7.7 | 9.0 | 5.4 |
| 127 | 1.1 | 431 | . 59 | 300 | 85 | 9 | . 4 | 678 | 7.4 | 8.0 | 17 |
| 144 | . 03 | 238 | . 32 | 180 | 0 | 14 | . 5 | 397 | 7.7 | 11.0 | 7.2 |
| 153 | . 03 | 107 | . 15 | 77 | 0 | 11 | . 2 | 170 | 7.9 | 10.5 | 2.1 |

[^2]Table 8.--Recommended limits of selected substances in water supplies
[Adapted from U.S. Public Health Service, 1962. The following chemical substances should not be present in a water supply in excess of the listed concentrations where, in the judgment of the reporting agency and the certifying authority, other more suitable supplies are or can be made available]

| Substance | Concentration, in |  |
| :---: | :---: | :---: |
|  | Milligrams per liter | Micrograms per liter |
| Chloride (C1)- | 250 |  |
| Dissolved solids | 500 |  |
| Fluoride (F) ${ }^{\text {l }}$ | 1.3 |  |
| Iron (Fe) | . 30 | 300 |
| Manganese (Mn) | . 05 | 50 |
| Nitrite-plus-nitrate as nitrogen ( $N)^{2}$ | 10 |  |
| Sulfate ( $\mathrm{SO}_{4}$ )- | 250 |  |

[^3]The concentration of dissolved iron, $6,000 \mu \mathrm{~g} / \mathrm{L}$ (micrograms per liter), exceeded the recommended limit in water from well SC 22-72-32AAB. Dissolved iron greater than $300 \mathrm{\mu g} / \mathrm{L}$ imparts a bitter taste to beverages, such as coffee and tea made using the water, and stains laundry and porcelain fixtures.

The concentration of dissolved nitrite plus nitrate of $14 \mathrm{mg} / \mathrm{L}$ exceeds the recommended limit in water from well SC 24-71-06BAA,a 29-ft deep windmill.

The concentrations of dissolved iron, 4,200 $\mu \mathrm{g} / \mathrm{L}$; manganese, $230 \mu \mathrm{~g} / \mathrm{L}$; and fluoride, $1.8 \mathrm{mg} / \mathrm{L}$, exceeded recommended limits in water from well NA 45-12-02DAA. Dissolved manganese greater than $50 \mu \mathrm{~g} / \mathrm{L}$ causes the same effects as dissolved iron greater than $300 \mu \mathrm{~g} / \mathrm{L}$. Dissolved fluoride greater than $1.3 \mathrm{mg} / \mathrm{L}$ can cause mottling of children's teeth.

Ground water in the basin is generally hard (as defined by Hem, 1970, p. 225). The hardness, expressed as calcium carbonate (table 7), in the samples analyzed ranged from 74 to $300 \mathrm{mg} / \mathrm{L}$. All but four of the samples have a hardness of $100 \mathrm{mg} / \mathrm{L}$ or greater. According to Hem (1970), the hardness of water to be used for domestic purposes becomes objectionable when the concentration is about $100 \mathrm{mg} / \mathrm{L}$.

According to J. M. K1ein, P. A. Emery, L. A. Hershey, and R. D. Penley (written commun., 1977), there are two different water-quality zones in the basin-fill aquifer. Generally, east of the axis of the valley the concentration of dissolved silica is greater than $25 \mathrm{mg} / \mathrm{L}$ and the concentrations of dissolved solids are greater than $200 \mathrm{mg} / \mathrm{L}$, while in the western part of the basin these constituents are generally less than these concentrations. The difference in water quality between these two zones is probably due to the different sources of the sediments in the two areas. The source of the sediments in the western one-half of the valley is the Sangre de Cristo Range and the source for the east side of the basin is the Wet Mountains.

Dissolved silica in ground water originates from dissolution of various silicate minerals or volcanic glass which the ground water contacts. Large areas of the Wet Mountains are covered with layers of volcanic rocks (fig. 2) that contain substantial proportions of readily dissolved silica. Sediments derived from these volcanic rocks are the probable source of the larger silica concentrations on the east side of the valley.

## Surface Water

Two surface-water samples were collected for water-quality analyses from a site along lower Grape Creek (p1. 1). The results of these analyses are listed in table 9 and indicate that, at the time the samples were collected, the substances analyzed for were within the recommended limits for drinking water set by the U.S. Public Health Service (1962) (table 8). Wentz (1974) lists temperature, pH , and specific-conductance data for several surface-water sites in the study area.

## Suitability of Water for Irrigation

The suitability of water for irrigation is dependent on several variables, the most important of which are the quality of the water, the type of soil and crops to be irrigated, and the irrigation practices used. A concentration of dissolved solids greater than $1,000 \mathrm{mg} / \mathrm{L}$ (Robinove and others, 1958) may affect crop growth indirectly through the effect on the physical condition of the soil, and directly by increasing the osmotic pressure of the soil solution and by an accumulation of toxic quantities of various ions within the plant. Thus, the ability of a plant to use water is diminished and a reduced crop yield results (Reeve, 1957, p. 175). Sodium buildup in the soil may reduce soil permeability, which limits air penetration to the root zone, leads to waterlogging, and reduces plant growth (Klein and Bingham, 1975).

A diagram used for classification of irrigation water, based on the sodium-adsorption ratio (SAR) and the specific conductance, is shown on figure 11. This diagram is a useful tool in predicting potential sodium and salinity hazards involved in the use of water for irrigation. The diagram has 16 different categories for classifying irrigation water. These categories range from low salinity (C1), low sodium (S1), which is water that can be
Table 9.--Chemical analyses of water from selected surface-water sites


[^4]

SALINITY HAZARD

Figure 11.-- Classification of irrigation water.
applied to nearly all types of soils and crops with little danger of harmful effects, to very high salinity (C4), very high sodium (S4), which is water suitable for irrigation use only under very limited conditions (table 10).

All of the water samples analyzed during the course of this study plot within the Sl sodium-hazard category and all but one plot within either the C1 or C2 salinity-hazard categories (fig. 11). This would indicate that both the surface and ground water have low-sodium hazard and low- to medium-salinity hazard. Within the study area, surface and ground water are generally safe to use for irrigation under most conditions.

## SUMMARY

The Wet Mountain Valley is an intermontane trough filled with unconsolidated deposits of Cenozoic age, and bordered by mountain ranges composed of volcanic rocks of Tertiary age and igneous, metamorphic, and sedimentary rocks of pre-Tertiary age. The valley includes an area of $540 \mathrm{mi}^{2}$ and is primarily drained by Grape Creek and Texas Creek. Average annual precipitation ranges from less than 16 in. to more than 40 in., generally increasing with land-surface altitude.

Ground water occurs in two different hydrologic areas within the study area--the mountainous area, which is composed chiefly of consolidated rock; and the valley-floor area, which is underlain by unconsolidated basin-fill sediments. In the mountainous area, well yields are highly unpredictable but are generally less than $10 \mathrm{gal} / \mathrm{min}$. In the basin-fill aquifer, yields of greater than $50 \mathrm{gal} / \mathrm{min}$ can generally be expected from wells located near the central part of the valley. In areas around the edge of the basin-fill aquifer, well yields of less than $50 \mathrm{gal} / \mathrm{min}$ are generally reported.

The basin-fill deposits cover an area of about $233 \mathrm{mi}^{2}$ and are the principal source of ground water in the study area. South of Westcliffe, this material is at least $6,700 \mathrm{ft}$ thick and the upper $1,200 \mathrm{ft}$ is composed of sand and gravel with interbedded and intergranular clay. There is an estimated 1.5 million acre-ft of ground water stored in the upper 100 ft of saturated basinfill sediments although considerably less than this amount could be developed from wells. Hydrographs of selected wells indicate that the volume of water stored in the basin-fill material is not changing.

Ground water occurs under both artesian and water-table conditions within the basin-fill aquifer. Artesian conditions exist, to some extent, throughout most of the basin-fill area and there are several flowing wells. The depth to the water table is less than 10 ft in an area of about $46 \mathrm{mi}^{2}$ along the central part of the valley and is less than 100 ft in most of the remainder of the valley.

Ground-water movement in the basin-fill aquifer is generally toward Grape and Texas Creeks. Ground water leaves the aquifer either by discharge to streams, discharge by wells, or as direct evapotranspiration from the water table. Underflow out of the basin is considered negligible.

Table 10.--Salinity and sodium classification for irrigation water [Adapted from U.S. Salinity Laboratory Staff (1954)]

SALINITY

| Class | Specific conductance, in micromhos per centimeter at $25^{\circ} \mathrm{C}$ | Comments |
| :---: | :---: | :---: |
| C1 (low)------ | Less than 250 | May be used for any crop on nearly all soils. |
| C2 (medium)---- | 250-750 | May be used if a moderate amount of leaching occurs. |
| C3 (high)------ | 750-2,250 | Should be used only on soils with better than adequate drainage and for plants with good salt tolerance. |
| c4 (very high)- | Greater than 2,250 | Not suitable for irrigation under ordinary conditions, but may be used on permeable soil with adequate drainage if water is applied in excess to provide considerable leaching. Very salt-tolerant plants should be selected. |

## SODIUM

Class
Comments
S1 (low)------- May be used for any crop on nearly all soils.
S2 (medium)---- May be used on coarse-textured (sandy) soils with good permeability; may present a moderate sodium problem in finetextured (clay) soils unless gypsum is present.
S3 (high)------ May present sodium problems in most soil types.
S4 (very high) - Not suitable for irrigation of any crops in any soil, except under special conditions.
Sodium-adsorption-ratio range for each class is dependent on specific conductance.

Streamflow in the area is typical of a high mountain valley with most runoff occurring in June and July as the result of snowmelt. Observed streamflow consists of runoff from precipitation, return flow from irrigation, discharge of ground water, and water released from reservoir storage. Continuous records of streamflow have been collected since 1924 at the Grape Creek near Westcliffe gaging station. Additionally, two continuous-record and four partial-record stations have been operated in the study area since August 1974. Analysis of the streamflow data for water years 1975 and 1976 shows that the average annual runoff for $1975-76$ at the seven stations ranged from about 420 to 24,240 acre-ft per year. Long-term annual runoff (1931-60) at these stations ranges from an estimated $0.02\left(\mathrm{ft}^{3} / \mathrm{s}\right) / \mathrm{mi}^{2}$ to an estimated $1.17\left(\mathrm{ft}^{3} / \mathrm{s}\right) / \mathrm{mi}^{2}$, generally increasing with station altitude. Using the data at two partial-record stations where natural-flow conditions prevailed, the magnitude, frequency, and depth of floods were estimated using basin characteristics and regionalized regression equations.

DeWeese Reservoir, located near the study area, has a storage capacity of 1,772 acre-ft. A11 of the other reservoirs within the study area have a combined storage capacity of about 215 acre-ft.

Generalized annual water budgets for two areas in the Wet Mountain Valley indicate that surface-water outflow is only 7 to 11 percent of the total water supply from precipitation and other sources. The remaining water is lost to the atmosphere by evapotranspiration. Ground-water underflow and changes in ground-water storage are considered insignificant in both water budgets.

The quality of both the surface and ground water is generally within the recommended limits for drinking water set by the U.S. Public Health Service (1962). The hardness of the ground water is generally greater than $100 \mathrm{mg} / \mathrm{L}$ as calcium carbonate. There are two different water-quality zones in the basin-fill aquifer. East of the axis of the valley, the concentration of dissolved silica is greater than $25 \mathrm{mg} / \mathrm{L}$ and the concentration of dissolved solids is greater than $200 \mathrm{mg} / \mathrm{L}$. In the western part of the valley, these constituents are generally less than these concentrations.

Both the surface and ground water are generally safe to use for irrigation. None of the samples analyzed indicated a potential sodium hazard and most indicated only a low- to medium-potential salinity hazard.

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## System of Numbering Water-Quality Sampling Sites

The system of numbering water-quality sampling sites is based on the grid system of latitude and longitude. The system provides the geographic location of the site and a unique number for each site. The number consists of 15 digits. The first six digits denote the degrees, minutes, and seconds of latitude; the next seven digits denote degrees, minutes, and seconds of longitude; and the last two digits are a sequential number for sites within a 1 -second grid. (In the event that the latitude-longitude coordinates for two or more sites are the same, sequential numbers " 01, " " 02 ," and so forth are assigned. See figure 12.)

## System of Numbering We11s

The well locations in this report are given numbers based on the U.S. Bureau of Land Management system of land subdivision, and show the location of the wells by quadrant, township, range, section, and position within the section. A graphic illustration of this method of well location is shown in figure 13. The first letter of the location number indicates which principal meridian governs the area in which the well is located; $S$ indicates the sixth principal meridian and $N$ indicates the New Mexico principal meridian. The second letter indicates the quadrant in which the well is located. Four quadrants are formed by the intersection of the base line and the principal meridian; A indicates the northeast quadrant, $B$ the northwest, $C$ the southwest, and D the southeast. The first number indicates the township; the second, the range; and the third, the section in which the well is located. The letters following the section number indicate the location of the well within the section. The first letter denotes the quarter section; the second, the quarter-quarter section; and the third, the quarter-quarter-quarter section. The letters are assigned within the section in a counterclockwise direction, beginning with $A$ in the northeast quarter. Letters are assigned within each quarter section and within each quarter-quarter section in the same manner. Where two or more locations are within the smallest subdivision, consecutive numbers, beginning with 1 , are added in the order in which the wells are inventoried. For example, SC23-72-15DAA indicates a well in the NE $\frac{1}{4} \mathrm{NE}_{\frac{1}{4}}^{4} \mathrm{SE}^{\frac{1}{4}} \mathrm{sec} .15, \mathrm{~T} .23 \mathrm{~S} ., \mathrm{R} .72 \mathrm{~W}$.

## System of Numbering Surface-Water Stations

Surface-water station numbers are assigned in downstream direction along the main stream. Station numbers on tributaries are assigned between station numbers on the main stream in the order in which those tributaries enter the main stream. Downstream-order station numbers are not assigned to miscellaneous sites where water-quality samples are collected or discharge measurements are made intermittently.


Coordinates for site A 381241105373101
B 381241105373102
C 381244105373301

Figure 12.-U.S. Geological Survey's system of numbering water-quality sampling sites.


Figure 13.-- System of numbering wells.

| SC 23-72-6AAB1 <br> (Altitude 7,930 feet above mean sea level) | ```Thick- ness, in feet``` | Depth below land surface, in feet |
| :---: | :---: | :---: |
| Soil, brown; contains scattered 1-inch rounded gravel | 3 | 3 |
| Gravel, clayey; contains rounded cobbles as large as 6 inches | 3 | 6 |
| Gravel, very fine, to very coarse sand; contains some medium gravel | 4 | 10 |
| Sand, very coarse, silty; contains some medium grave | 10 | 20 |
| Sand, very coarse, arkosic; contains some reddish-brown c | 10 | 30 |
| Clay, reddish-brown | 5 | 35 |
| Gravel, fine to very fine, clay | 10 | 45 |
| Clay, reddish-bro | 5 | 50 |
| Gravel, fine to very fine, claye | 5 | 55 |
| Gravel, fine, well-sorted, clay | 5 | 60 |
| Sand, very coarse, clayey, grading to well-sorted fine gravel | 10 | 70 |
| Clay, reddish-brown, sandy, grading to clay; contains thin yellowish-gray, hard, platy bentonite(?) stringers-------- | 10 | 80 |
| Gravel, very fine, to coarse sand, clayey; contains cemented beds in bottom 10 feet | 25 | 105 |
| Gravel, very fine, to sand, very coarse, c | 15 | 120 |
| Gravel, very fine to fine, clayey | 40 | 160 |
| Gravel, grading from coarse to very coarse sand; contains clay streaks and some intergranular <br> clay-------------------------- | 20 | 180 |
| Gravel, very fine to fine, and sand, clayey; contains some clean beds | 30 | 210 |
| Clay, grayish-yellow, and interbedded sand and grave | 20 | 230 |
| Sand and very fine gravel, some clay | 20 | 250 |
| Sand, very coarse, to gravel, very fine, clayey; contains cemented zone from 255 to 260 feet | 20 | 270 |
| Sand, very coarse to coarse, grading to fine gravel | 10 | 280 |
| Gravel, very fine to fine, and sand grading to very fine, silty gravel | 20 | 300 |
| Sand, silty, poorly sorted | 25 | 325 |
| Gravel and sand, clayey- | 10 | 335 |

## Geologic Log of Test Hole SC 23-72- 6AAB1--Continued

| SC 23-72-6AAB1 <br> (Altitude 7,930 feet above mean sea level) | ```Thick- ness, in feet``` | ```Depth below land surface, in feet``` |
| :---: | :---: | :---: |
| Sand, coarse to medium, clayey; contains cemented(?) sand and gravel | 40 | 375 |
| Sand, medium, to very fine gravel, clayey; clean(?) from 375 to 380 feet | 20 | 395 |
| Sand, very clayey, grading to clay, san | 20 | 415 |
| Sand and interbedded | 20 | 435 |
| Clay, sandy; contains some very fin | 20 | 455 |
| Sand, coarse, clayey, grading to clay, containing very fine gravel | 20 | 475 |
| Sand, very coarse, grading to sandy | 20 | 495 |
| Sand, medium, to very fine gravel, grading | 20 | 515 |
| Sand, medium to coarse, grading to sandy clay | 20 | 535 |
| Clay, sandy, and | 20 | 555 |
| Sand, medium to coarse; contains clay | 20 | 575 |
| Sand, poorly sorted, some clayey and reddish-brown clay streaks; some caliche bottom 5 feet | 40 | 615 |
| Sand, medium to very coarse, and very fine gravel; contains clay streaks- | 20 | 635 |
| Sand, medium to coarse, clayey; contains beds of yellowishgray bentonitic(?) clay | 20 | 655 |
| Sand, very coarse, clean, grading to poorly sorted clayey sand- | - 20 | 675 |
| Sand, coarse, to very fine gravel, clay | 50 | 725 |
| Sand, silty to clayey, cemented(?); small amount of reddishbrown clay and yellowish-gray, bentonitic(?) clay---------- | 10 | 735 |
| Sand, silty to clayey, loosely cemented, grading to reddishbrown clay- | 15 | 750 |
| Clay, reddish-brown to greenish-y | 10 | 760 |
| Clay, sandy, greenish-yellow to pink; contains some caliche-- | 15 | 775 |
| Clay, reddish-brown, sandy, grading to varicolored clays | 20 | 795 |
| Sand and very fine gravel, clay-filled | 20 | 815 |
| Sand, medium to coarse, clayey, grading to sandy clay | 20 | 835 |


| SC 23-72- 6AAB1 <br> (Altitude 7,930 feet above mean sea level) | ```Thick- ness, in feet``` | Depth below land surface, in feet |
| :---: | :---: | :---: |
| Sand, fine to medium; contains some clay grading to very fine gravel and coarse sand- | 20 | 855 |
| Sand, fine, clayey, grading to clean coarse | 20 | 875 |
| Sand, medium to coarse, grading to clayey sand and some tightly cemented conglomerate- | 20 | 895 |
| Sand and gravel, conglomeratic; some interbedded clay | 20 | 915 |
| Sand to very fine gravel; contains loosely cemented stringers-- | - 20 | 935 |
| Sand, clayey; contains some tightly cemented sand and gravel beds | - 20 | 955 |
| Sand, poorly sorted to claye | 20 | 975 |
| Sand, medium to very coarse, less clayey than | 20 | 995 |
| Sand, very coarse, grading to very fine, clayey gravel----.---- | - 20 | 1,015 |
| Sand and fine gravel grading to sand and medium gravel; contains clay stringers | 20 | 1,035 |
| Sand and very fine gravel grading to tightly cemented gravel--- | - 20 | 1,055 |
| Sand, fine to very coarse; contains some reddish-brown clay and cemented beds | 20 | 1,075 |
| Gravel, very fine to medium; contains cemented beds and sandy clay- | - 40 | 1,115 |
| Gravel, fine to very fine, and sand, and interbedded caliche, brown clay, and sandstone | - 20 | 1,135 |
| Sand, fine to very coarse, silty and claye | 20 | 1,155 |
| Sand and interbedded reddish-brown clay | 25 | 1,180 |
|  | 20 | 1,200 |

Records of Selected Wells
[Depth to water: F, Flowing.
Major aquifer codes: 02, Basin fill; E7, Dry Union Formation; E8, Santa Fe Formation or
Group; E9, Tertiary, igneous, undivided; F1, Precambrian, undifferentiated.
Type lift: C, Centrifugal; J, Jet; N, None; P, Piston; S, Submersible; T, Turbine.
Power: E, Electric; G, Gas, diesel, naturai or LP gas engine; H, Hand; N, None; W, Wind.
Use of water: H, Domestic; I, Irrigation; P, Public supply; S, Stock; U, Unused]


Records of Selected Wells--Continued

| Site number on figure 2 |  | Local well number | Year drilled | Alti- <br> tude of land surface (ft) | Well depth (ft) | Depth to water (ft) | Date water level measured (M-Y) | Major aquifer | Yield (gal/ min) | Drawdown (ft) | ```Casing diam- eter (in.)``` | Type <br> lift | Power | Use of water |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 61 | SC | 22-73-27BAD | 1964 | 8,040 | 128 | 12 | 12-64 | 02 | 30 | --- | 7 | - | E | H |
| 62 | SC | 22-73-28BBC | 1953 | 8,238 | 89 | -- | ----- | 02 |  | --- | 6 | C | - | H |
| 63 | SC | 22-73-28BDD | ---- | 8,240 | 82 | 12 | 8-71 | 02 | ------- | --- | 8 | $J$ | E | H |
| 64 | SC | 22-73-35CDB | ---- | 8,182 | 13 | 4 | 4-71 | 02 | ------- | --- | 18 | $J$ | E | H |
| 65 | SC | 22-73-36DAA | ---- | 7,990 | 26 | 4 | 11-70 | E8 | ------- | --- | 6 | P | W | S |
| 66 | SC | 22-73-36DCC | ---- | 8,020 | 103 | 3 | 11-70 | E8 | ------- | --- | 12 | $N$ | $N$ | 1 |
| 67 | SC | 23-71- 4DCC | 1861 | 8,635 | 18 | 5 | 4-71 | 02 | ------- | --- | 60 | - | - | S |
| 68 | SC | 23-71-9ABC | 1971 | 8,650 | 110 | 70 | 4-71 | F1 | 2 | --- | 6 | $N$ | N | H |
| 69 | SC | 23-71-9DBB | ---- | 8,660 | 300 | 71 | 4-71 | F1 | . 5 | --- | 6 | $N$ | N | H |
| 70 | SC | 23-72-1AAB | 1953 | 8,400 | 13 | 9 | 4-71 | 02 |  | --- | 48 | P | W | S |
| 71 | SC | 23-72- 4CDB | 1958 | 7,975 | 30 | 3 | 11-70 | 02 | ------- | --- | 7 | $J$ | E | H |
| 72 | SC | 23-72-5AAB1 | 1870 | 7,905 | 16 | 13 | 11-70 | 02 | ------- | --- | -- | P | E | H |
| 73 |  | 23-72-5ABB2 | ---- | 7,895 | 12 | 4 | 11-70 | 02 | ------- | --- | 60 | - | - | S |
| 74 |  | 23-72- 6AAB1 | 1971 | 7,930 | 1,200 | F | 7-71 | E8 | ------- | --- | 5 | N | $N$ | U |
| 75 | SC | 23-72-6AAB2 | 1974 | 7,930 | 92 | 16 | 5-74 | 02 | ------- | --- | 3 | N | $N$ | U |
| 76 | SC | 23-72-6BDD | ---- | 7,995 | 34 | 1 | 11-70 | 02 |  | --- | 6 | $p$ | W | S |
| 77 |  | 23-72-6CBD | -- | 8,043 | 40 | 4 | 11-70 | 02 |  | -- | 6 | P | - | S |
| 78 |  | 23-72-7ADA1 | --- | 8,000 | 16 | 4 | 11-70 | 02 |  | --- | 60 | P | H | S |
| 79 | SC | 23-72-7ADA2 | - | 7,995 | 25 | 3 | 11-70 | 02 |  | --- | 8 | P | E | S |
| 80 | SC | 23-72-7BCC1 | -- | 8,130 | 180 | 10 | 10-70 | E8 | ------- | --- | 12 | T | E | 1 |
| 81 |  | 23-72-7BCC2 | --- | 8,132 | 22 | 3 | 8-71 | 02 |  | --- | 6 | $N$ | N | U |
| 82 |  | 23-72-7CBB1 | --- | 8,142 | 28 | 1 | 10-70 | 02 |  | --- | 12 | N | N | U |
| 83 | SC | 23-72-7CBB2 | - | 8,142 | 115 | 1 | 10-70 | 02 |  | --- | 12 | $N$ | N | - |
| 84 | SC | 23-72-7CCC1 | ---- | 8,180 | 120 | 2 | 10-70 | E8 | ------- | --- | -- | T | E | 1 |
| 85 | SC | 23-72-7CCC2 | ---- | 8,175 | 30 | 1 | 11-70 | 02 | ------- | --- | 8 | $N$ | N | U |
| 86 | SC | 23-72-8CCC | 1962 | 8,035 | 60 | 3 | 11-70 | E8 | 10 | --- | 6 | $N$ | $N$ | S |
| 87 | SC | 23-72-10BBA | 1960 | 8,070 | 115 | 85 | 12-70 | E8 | ------- | --- | 6 | P | W | S |
| 88 | SC | 23-72-10BDD | 1952 | 8,000 | 44 | 12 | 12-70 | 02 | ------- | --- | 8 | $J$ | E | H |
| 89 | SC | 23-72-10DDD | ---- | 8,020 | 54 | 10 | 10-70 | E8 | - | --- | 12 | T | - | 1 |
| 90 |  | 23-72-14BBD | ---- | 8,060 | 52 | 44 | 12-70 | 02 | ------- | -- | 40 | P | E | S |


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|  | 동ㅇㅇㅇㅇ <br>  |  |  |  | $\begin{aligned} & \text { 오욲욱 } \\ & \frac{1}{-}=\frac{1}{=} \end{aligned}$ |  |
| GNMNN | amon＝0 | Nナニino | $N ナ ル \underset{\sigma}{n}$ | $\cdots \mathrm{mN}$ | N以NNN | $\widehat{O}^{\infty}$ |
| nino | $\underset{\sim}{\sim}{\underset{N}{N}}_{\infty}^{\infty} m$ | m육 in | ㅍㅇㅇㅇㅇ́ㅇ | 웅ㅇㅇ운 | 응ㅇN N | MoNN |
|  －す 月 Nin $\infty^{\circ} \infty^{n}$ へヘ $\infty^{\circ} \infty^{n}$ | のペべベ゚ <br>  $\infty^{n} \infty^{n} \infty^{n} \infty^{n}$ |  | －min in ONNN $\infty^{n} \infty^{n} \infty^{n} \infty^{n} \infty^{n}$ |  |  $N-\infty \mathrm{m}$ $\infty \infty \infty \infty$ |  |
| ：$: 1: \mid c$ | ন： | ： |  | ； | 式：Oㅇ： |  |
|  |  |  |  |  |  |  |
| ưusu | úusu | 岛岛品 | cusus | 岛鸭先 | Uu心式 | ưưu |
|  | ぷふळの옹 | 으웅 | 응ㅇㅇㅇㅇㅇㅇㅇㅁㅁㅁ | Nッツニ늗 |  | $\underset{\sim}{N} \underset{N}{N} \underset{\sim}{N}$ |

Records of Selected Wells--Continued

| Site number on figure 2 | Local well number | $\begin{gathered} \text { Year } \\ \text { drilled } \end{gathered}$ | Altitude of land surface (ft) | $\begin{aligned} & \text { Well } \\ & \text { depth } \\ & (f t) \end{aligned}$ | Depth <br> to <br> water (ft) | $\begin{aligned} & \text { Date } \\ & \text { water } \\ & \text { level } \\ & \text { meas- } \\ & \text { ured } \\ & (M-Y) \end{aligned}$ | Major aquifer | Yield <br> (gal/ <br> min) | Drawdown (ft) | $\begin{gathered} \text { Casing } \\ \text { diam- } \\ \text { eter } \\ \text { (in.) } \end{gathered}$ | Type lift | Power | Use of water |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 126 | SC 24-71-5BCA | ---- | 8,320 | 99 | 90 | 12-70 | E8 | ------- | --- | -- | $P$ | W | S |
| 127 | SC 24-71-6BAA | --- | 8,240 | 29 | 25 | 4-71 | E7 | ------- | --- | 36 | P | W | S |
| 128 | SC 24-71-7ACD | ---- | 8,358 | 142 | 138 | 4-71 | E. 7 | -------- | --- | 6 | P | W | S |
| 129 | SC 24-71-14DCA | 1965 | 8,721 | 70 | 24 | 10-70 | E7 | 10 | 5 | 5 | $P$ | W | S |
| 130 | SC 24-71-15BCC | ---- | 8,480 | 228 | 219 | 4-71 | E8 | --...-..- | --- | -- | $P$ | W | S |
| 131 | SC 24-71-17ABA | --- | 8,438 | --- | --- | ------ | E7 | -------- | --- | 6 | P | W | S |
| 132 | SC 24-71-17CBA | ---- | 8,485 | --- | 258 | 4-71 | E 7 | - | --- | 6 | P | W | S |
| 133 | SC 24-71-18DAA | --- | 8,465 | 240 | --- | --- | E7 | ------- | --- | 6 | P | W | S |
| 134 | SC 24-71-19BDD | 1931 | 8,571 | 332 | 320 | 4-71 | E7 | ------- | --- | 5 | $N$ | N | H |
| 135 | SC 24-71-22BBC | ---- | 8,585 | --- | 309 | 4-71 | E8 | ------- | --- | 6 | P | W | S |
| 136 | SC 24-71-25CBA | 1966 | 9,038 | 400 | 165 | 6-66 | E8 | 2 | 135 | 5 | P | W | S |
| 137 | SC 24-71-35BAB | --- | 8,875 | -- | --- | ------ | 02 | ------- | --- | -- | P | W | S |
| 138 | SC 24-72-4DBB | 1964 | 8,320 | 142 | 4 | 11-70 | 02 | 20 | 0 | 6 | C | $E$ | S |
| 1.39 | SC 24-72-5ABB | 1964 | 8,545 | 150 | 92 | 11-70 | E8 | ------- | --- | 6 | P | G | S |
| 140 | SC 24-72-10CCC | 1969 | 8,440 | 220 | 180 | 7-69 | E8 | 10 | 10 | 7 | S | E | H |
| 141 | SC 24-72-15AAD | 1959 | 8,495 | 310 | 295 | 12-59 | 02 | 5 | 2 | 6 | P | W | S |
| 142 | SC 24-72-26BAC | - | 8,755 | 387 | Dry | 4-71 | E8 | -------- | --- | 6 | P | G | - |
| 143 | SC 24-72-28CAA | --- | 8,170 | 12 | 5 | 11-70 | 02 | ------- | --- | 36 | C | E | H |
| 144 | NA 45-12-2DAA | --- | 8,835 | 110 | - | ------ | -- | ------- | --- | 6 | S | E | H |
| 145 | NA 46-12-3DDD | 1954 | 7,650 | 150 | $F$ | 10-70 | E8 | -------- | --- | 5 | T | - | 1 |
| 146 | NA 46-12-10BDD | ---- | 7,860 | --- | 143 | 5-74 | 02 | --.----- | --- | 6 | P | W | S |
| 147 | NA 46-12-10DCB | - | 7,880 | 145 | 133 | 5-74 | 02 | -------- | --- | 6 | P | W | S |
| 148 | NA 46-12-11ACC | ---- | 7,620 | 130 | 51 | 10-70 | E8 | ----- | --- | -- | T | - | 1 |
| 149 | NA 46-12-11CDA | --- | 7,749 | 196 | 24 | 10-70 | E8 | - | --- | 6 | P | W | S |
| 150 | NA 46-12-12DAC | -- | 7,620 | 104 | $F$ | 10-70 | E8 | 20 | --- | 8 | - | - | I |
| 151 | NA 46-12-12DBB1 | - | 7,630 | 161 | $F$ | 11-70 | E8 | ------- | - | 8 | $N$ | N | H |
| 152 | NA 46-12-12DBB2 | - | 7,640 | 159 | $F$ | 10-70 | E8 | 2 | --- | 8 | - | H | H |
| 153 | NA 46-12-12DBC | ---- | 7,650 | 184 | $F$ | 10-70 | E8 | 40 | --- | 8 | - | - | 1 |
| 154 | NA 46-12-12DBD1 | ---- | 7,600 | 89 | $F$ | 10-70 | E8 | 40 | - | 10 | $N$ | $N$ | I |
| 155 | NA 46-12-120BD2 | ---- | 7,615 | 121 | $F$ | 10-70 | E8 | 9 | --- | 10 | $N$ | $N$ | I |



## Specific Conductance and Temperature of Water Data from

 Selected Surface-Water Stations| Site number on figure 2 | U.S. Geological Survey station number | Station name | Date of sample | Temperature (degrees Celsius) | Specific conductance ( $\mu \mathrm{mho} / \mathrm{cm}$ at $25^{\circ} \mathrm{C}$ ) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 1----- | 07093980 | Lake Creek near Hillside, Colo. | 7-31-74 | 11.5 |  |
|  |  |  | 8-19-74 | 12.5 |  |
|  |  |  | 9-16-74 | 9 |  |
|  |  |  | 10-16-74 | 7 |  |
|  |  |  | 11-15-74 | 3 |  |
|  |  |  | 12-18-74 | 0 |  |
|  |  |  | 4-18-75 | 2 | 150 |
|  |  |  | 5-21-75 | 11.5 |  |
|  |  |  | 6-13-75 | 10 |  |
|  |  |  | 8-6-75 | 10.5 |  |
|  |  |  | 9- 9-75 | 10 | 119 |
|  |  |  | 10-7-75 | 8 | 130 |
|  |  |  | 11-11-75 | 1.5 |  |
|  |  |  | 12-23-75 | 1.5 | 123 |
|  |  |  | 2-3-76 | 2.5 |  |
|  |  |  | 4-8-76 | 7 | 122 |
|  |  |  | 5-5-76 | 9.5 | 127 |
|  |  |  | 8-13-76 | 10.5 | 120 |
|  |  |  | 9-17-76 | 9.5 | 125 |
| 2----- | 07094530 | Grape Creek near Bradford, Colo. | 7-31-74 | 19 |  |
|  |  |  | 8-19-74 | 12.5 |  |
|  |  |  | 10-16-74 | 3 |  |
|  |  |  | 11-15-74 | 0 |  |
|  |  |  | 12-18-74 | 0 |  |
|  |  |  | 5-20-75 | 9 |  |
|  |  |  | 6-13-75 | 6.5 |  |
|  |  |  | 7-10-75 | 9.5 |  |
|  |  |  | 8-6-75 | 11.5 |  |
|  |  |  | 9-9-75 | 5.5 | 101 |
|  |  |  | 10-7-75 | 9.5 | 155 |
|  |  |  | 11-11-75 | 0 |  |
|  |  |  | 12-23-75 | 0.5 |  |
|  |  |  | 2-3-76 | 0.5 |  |
|  |  |  | 4-8-76 | 2.5 | 132 |
|  |  |  | 5-5-76 | 5.5 | 101 |
|  |  |  | 6-30-76 | 14 | 115 |
|  |  |  | 8-13-76 | 12.5 | 139 |
|  |  |  | 9-17-76 | 20 | 115 |

Specific Conductance and Temperature of Water Data from
Selected Surface-Water Stations--Continued

| Site number on figure 2 | U.S. Geological Survey station number | Station name | Date of sample | Temperature (degrees Celsius) | Specific conductance ( $\mu \mathrm{mho} / \mathrm{cm}$ at $25^{\circ} \mathrm{C}$ ) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 4----- | 07094600 | South Colony Creek near Westcliffe, Colo. | 8-1-74 | 14 |  |
|  |  |  | 8-19-74 | 10.5 |  |
|  |  |  | 9-16-74 | 6.5 |  |
|  |  |  | 10-16-74 | 3 |  |
|  |  |  | 11-15-74 | 0 |  |
|  |  |  | 4-17-75 | 0.5 | 140 |
|  |  |  | 5-20-75 | 4.5 |  |
|  |  |  | 6-13-75 | 6.5 |  |
|  |  |  | 7-10-75 | 7.5 |  |
|  |  |  | 8-6-75 | 9 |  |
|  |  |  | 9- 9-75 | 9 | 95 |
|  |  |  | 10-7-75 | 6.5 | 100 |
|  |  |  | 11-11-75 | 0 |  |
|  |  |  | 12-23-75 | 0 | 122 |
|  |  |  | 2-3-76 | 0.5 |  |
|  |  |  | 4-8-76 | 2 | 116 |
|  |  |  | 5-5-76 | 3.5 | 91 |
|  |  |  | 6-11-76 | 8 | 70 |
|  |  |  | 7-12-76 | 13.5 | 70 |
|  |  |  | 8-13-76 | 8.5 | 90 |
|  |  |  | 9-17-76 | 10.5 | 105 |
| 5----- | 07094700 | Antelope Creek near Rosita, Colo. |  | 30.5 |  |
|  |  |  | 4-17-75 | 10.5 | 310 |
|  |  |  | 5-20-75 | 9 |  |
|  |  |  | 6-13-75 | 12 |  |
|  |  |  | 7-10-75 | 14.5 |  |
|  |  |  | 8-6-75 | 16 |  |
|  |  |  | 9- 9-75 | 12 | 360 |
|  |  |  | 11-11-75 | 0 |  |
|  |  |  | 2-3-76 | 0 |  |
|  |  |  | 4-8-76 | 9.5 | 362 |
|  |  |  | 5-5-76 | 10 | 370 |
|  |  |  | 6-30-76 | 23 | 370 |

Specific Conductance and Temperature of Water Data from
Selected Surface-Water Stations--Continued

| Site number on figure 2 | U.S. Geological Survey station number | Station name | Date of sample | Temperature (degrees Celsius) | Specific conductance ( $\mu \mathrm{mho} / \mathrm{cm}$ at $25^{\circ} \mathrm{C}$ ) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 6----- | 07094800 | Venable Creek near Westcliffe, Colo. | 7-31-74 | 10.5 |  |
|  |  |  | 8-19-74 | 9.5 |  |
|  |  |  | 9-16-74 | 5.5 |  |
|  |  |  | 10-16-74 | 3.5 |  |
|  |  |  | 5-21-75 | 6 |  |
|  |  |  | 6-13-75 | 8 |  |
|  |  |  | 7-10-75 | 9 |  |
|  |  |  | 8-6-75 | 10 |  |
|  |  |  | 9- 9-75 | 9 | 138 |
|  |  |  | 10-7-75 | 6 | 70 |
|  |  |  | 11-11-75 | 0 |  |
|  |  |  | 12-23-75 | 0 | 170 |
|  |  |  | 2-3-76 | 0 |  |
|  |  |  | 4-8-76 | 1 | 74 |
|  |  |  | 5-5-76 | 6 |  |
|  |  |  | 6-11-76 | 7 | 110 |
|  |  |  | 6-30-76 | 11 | 120 |
|  |  |  | 8-13-76 | 9.5 | 140 |
|  |  |  | 9-17-76 | 9 | 160 |
| 7----- | 07094900 | Middle Taylor Creek near Westcliffe, Colo. | 8-1-74 | 16 |  |
|  |  |  | 8-19-74 | 15.5 |  |
|  |  |  | 9-16-74 | 7.5 |  |
|  |  |  | 10-16-74 | 9 |  |
|  |  |  | 11-26-74 | 0 | 240 |
|  |  |  | 1-29-75 | 0 |  |
|  |  |  | 6-13-75 | 13 |  |
|  |  |  | 8-6-75 | 17.5 |  |
|  |  |  | 9- 9-75 | 13.5 | 197 |
|  |  |  | 10-7-75 | 10.0 | 215 |
|  |  |  | 10-16-75 | 7 | 218 |
|  |  |  | 12-23-75 | 0 | 220 |
|  |  |  | 2-3-76 | 0 |  |
|  |  |  | 4-8-76 | 0.5 | 215 |
|  |  |  | 5-5-76 | 6 | 150 |
|  |  |  | 5-11-76 | 7.5 | 125 |
|  |  |  | 6-11-76 | 10.5 | 105 |
|  |  |  | 6-30-76 | 17 | 155 |
|  |  |  | 7-12-76 | 12 | 50 |
|  |  |  | 8-13-76 | 13 | 185 |
|  |  |  | 9-17-76 | 10 | 210 |



OPEN-FILE REPORT
MAP SHOWING LOCATION OF WELLS, SURFACE-WATER GAGING STATIONS, AND POTENTIOMETRIC SURFACE OF THE BASIN-FILL AQUIFER, WET MOUNTAIN VALLEY, PARTS OF CUSTER AND FREMONT COUNTIES, COLORADO


[^0]:    Total
    44.53

[^1]:    ${ }^{1}$ Refers to sites shown on figure 2.
    ${ }^{2}$ Refers to available record from which flow characteristics were

[^2]:    ${ }^{1}$ See figure 2.

[^3]:    ${ }^{1}$ The recommended control limit for fluoride concentrations is variable and dependent upon the annual average of maximum daily air temperature.
    ${ }^{2}$ In areas in which the nitrate concentration of water is known to be in excess of the listed concentration, the public should be warned of the potential danger of using the water for infant feeding.

[^4]:    ${ }^{1}$ See figure 2

