A DEMONSTRATION OF MITIGATION OF ENVIRONMENTALLY-ASSISTED CRACKING BY THE APPLICATION OF A TENSILE OVERLOAD

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A DEMONSTRATION OF MITIGATION OF
ENVIRONMENTALLY-ASSISTED CRACKING
BY THE APPLICATION OF A TENSILE OVERLOAD

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ABSTRACT

Environmentally-assisted cracking (EAC) of low-alloy steels in elevated temperature aqueous environments is readily observed in many laboratory experiments conducted in autoclaves, yet the observation of EAC in actual components operating in the same environments is quite rare. Mass transport of sulfides from the crack enclave by diffusion and convection occurring in operating components provides one plausible explanation to this apparent paradox. Another contribution to EAC mitigation may also arise from the non-constant stress amplitudes typical for many operating components. This paper provides a demonstration of how a single tensile overload to 40-percent above a steady-state maximum fatigue stress can retard subsequent crack growth at the steady-state level for a sufficient period of time that diffusional mass transport can reduce the crack-tip sulfide concentration to a level below that necessary to sustain EAC.

INTRODUCTION

Environmentally-assisted cracking (EAC) of low-alloy steels in high-temperature aqueous environments typical of those employed in light-water reactor (LWR) systems has been a subject of considerable interest since the pioneering work of Kondo et al (1972a, 1972b) demonstrated significantly higher fatigue crack propagation (FCP) rates in water than would be expected in an air environment under similar conditions. A large number of research studies have been directed at the EAC issue, and the results of these studies have been summarized in a series of review articles (e.g., Tice, 1985; Bullock 1989; James, 1995a) each one of which represents an update in the current state of knowledge. The basic cause-and-effect relationship for EAC has been known for several years, and considerable progress has been made in quantifying the conditions under which EAC will, or will not, occur in a given situation. Quantifying the conditions under which EAC will occur is an important consideration because experience has shown that EAC occurs rather easily in autoclave experiments, yet the phenomenon is clearly not endemic amongst commercial LWR systems worldwide.

This paper will briefly discuss some of the mitigating factors that help explain why the occurrence of EAC is a relatively rare event in actual LWR operating systems. However, the main objective of this paper is to describe a demonstration of an additional possible mitigating factor: the mitigation of EAC by the application of a tensile overload.

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REVIEW OF EAC CAUSES AND MITIGATION

It has been recognized EAC occurs in a low-alloy steel undergoing cyclic loading in an aqueous environment when the concentration of hydrogen sulfide at the crack tip reaches a critical level; probably about $5 \pm 2$ ppb $H_2S$ (VanDerSluys, 1993). EAC manifests itself by exhibiting FCP rates considerably higher (sometimes by a factor of 40-100) than would be expected in an air environment under the same conditions.

The occurrence or non-occurrence of EAC is essentially a function of the local crack tip balance between the competing processes of sulfide supply and sulfide loss. In lieu of water-borne sulfur contamination, the only source of sulfides to the crack tip is the growing crack itself, which intersects metallurgical sulfide inclusions ($MnS$, $FeS$, etc.) embedded within the steel. These inclusions then dissolve in the aqueous crack-tip environment (Matsushita et al., 1991) and supply sulfide ions to the crack tip. Sulfides can be removed from the crack-tip region by any one, or more, of four mass-transport processes: 1) diffusion due to a sulfide concentration gradient from the crack tip to the crack mouth (e.g., Turnbull, 1982, 1992; Wire, 1996), 2) ion migration due to an electrochemical corrosion potential (ECP) gradient from the crack tip to the crack mouth (e.g., Turnbull, 1982), 3) fatigue "pumping" or advection due to cyclic motion of the crack walls (e.g., Hartt, 1978; Turnbull, 1983), and 4) convective transport induced within the crack enclave by an external (to the crack) stream flow (e.g., James, 1995b, 1997a). Not all of these mass-transport processes play a significant role in every case of corrosion-fatigue crack growth. For example, the second process, ion migration, is probably an important factor in the case of oxygenated BWR environments, where a significant ECP gradient can exist between the crack tip and the crack mouth. Potential gradients between the crack tip and crack mouth are much smaller for low-oxygen PWR environments (Gabetta, 1987). The third mass-transport process, fatigue "pumping", is likely to be a factor only at relatively high cyclic frequencies, and the fourth process, convective mass transport, would not be an important consideration under quasi-stagnant flow conditions. Note, however, that the first mass-transport process, diffusion, would be operative any time that a sulfide concentration gradient exists between the crack tip and the crack mouth. In fact, diffusion was essentially the only mass-transport operative in the experiment to be described herein because it was conducted in low-oxygen water at low cyclic frequencies under quasi-stagnant flow conditions.

Many corrosion-FCP experiments exhibit EAC at, or near, the start of the experiment. Combrade et al. (1988) have pointed out that the majority of specimens to be used in corrosion-FCP experiments are precracked in an air environment prior to insertion into the autoclave, and that the undissolved sulfide inclusions on the crack flanks can predispose the specimen toward the early exhibition of EAC. In many cases, EAC then persists throughout the entire experiment because the high EAC-related FCP rates continue to expose embedded sulfide inclusions at a rate sufficient to ensure that crack-tip sulfide concentrations exceed the critical level; i.e., a high rate of sulfide supply to the crack tip. Wire and Li (1996) have shown that even with high-sulfur steels in quasi-stagnant water, EAC does not initiate from an initially "clean" crack (i.e., $H_2S << "critical"$ concentration) until the average crack velocity, $V$, exceeds a critical level, $V_{in}$, and maintains that velocity for a minimum critical crack extension, $\Delta a_{crit}$; i.e.

$$V \geq V_{in} = (5 \times 10^{-7})(2.54/a)$$

Eq. [1a]  

and

$$\Delta a \geq \Delta a_{crit} = 0.33 \text{ mm}$$

Eq. [1b]
where the dimensions of $V$ and $a$ are mm/s and mm, respectively. In this context, the crack velocity is defined as the crack extension for an increment of time divided by the total time associated with that cracking increment. The total time includes all time spent with stresses increasing, stresses decreasing, plus hold or dwell periods. Wire and Li conducted most of their experiments on specimens having an initial precrack depth (from the notch) of 2.54 mm. Since diffusional mass transport, and hence the EAC Initiation Velocity $V_{\text{init}}$, is linearly related to the crack depth, the normalizing term $(2.54/a)$ was incorporated into Equation [1a].

The EAC Initiation Criterion of Equation [1] applies to the initiation of EAC in high-sulfur steels in a quasi-stagnant low-oxygen environment; i.e., diffusional mass transport is the only operative process for removing sulfides from the crack tip. Similar considerations can be applied to the cessation of an operative case of EAC in high-sulfur steels in a quasi-stagnant low-oxygen environment. Other scenarios for the cessation of an operative case of EAC include convective mass transport (James, 1995b, 1996) as well as the present study on tensile overload mitigation.

**EXPERIMENTAL PROCEDURES**

The steel utilized in this experiment was a high-sulfur heat of ASTM A508-2 (Heat 15812). This heat has been employed in previous experiments on corrosion-FCP in low-oxygen aqueous environments, and readily exhibits EAC at temperatures of 243°C (James, 1994) and 288°C (Van Der Sluys and Emanuelson, 1993) when special measures are not taken to reduce the crack-tip sulfide concentration prior to commencing fatigue testing. The specimen for the present study, as well as those for the previous studies, was machined from the reactor pressure vessel nozzle that was removed from an unfinished commercial nuclear plant. The chemical composition and room temperature mechanical properties of this heat are given in Tables I and II, respectively.

The specimen employed was a "2T" compact tension specimen ($W=5.08$ mm). Crack lengths were inferred from compliance measurements using an in-situ displacement transducer and the standard compliance relationship of Saxena and Hudak (1978). Stress intensity factors, $K$, were calculated using the standard relationship given by Saxena and Hudak.

Deaerated water containing about 40 cc H$_2$/kg H$_2$O was employed in this study. Dissolved O$_2$ was therefore generally less than 10 ppb. The room temperature pH was in the range 10.2 to 10.3, while the calculated** pH at the test temperature of 243°C was 6.8. The room temperature conductivity averaged about 50 $\mu$S/cm. The ECP was measured throughout the experiment using a Ag/AgCl reference electrode. The measurements were corrected to standard hydrogen electrode (SHE) conditions using the relationship of Macdonald et al. (1979). These results are plotted in Figure 1. The mean ECP value of -0.728 V$_{\text{SHE}}$ is in good agreement with earlier measurements in similar environments at 243°C (James, 1994, 1997a, 1997b, 1997c).

The specimen was cyclically-loaded by a computer-controlled servo-hydraulic apparatus operating in the constant-$AK$ mode. A "positive sawtooth" loading waveform was utilized with 85-percent load-rise time, 15-percent load-fall time, and no dwell period within or between fatigue cycles. With the exception of the intentionally-applied tensile overloads, the testing was done under "steady-state"
cycling conditions: $K_{\text{max}} = 21.98 \text{ MPa}\sqrt{\text{m}}$ (20 ksi/in$^\frac{1}{2}$), $R = \frac{K_{\text{min}}}{K_{\text{max}}} = 0.5$, and a cyclic frequency of 0.0167 Hz with a load-rise time of 51 seconds.

RESULTS AND DISCUSSION

It has been recognized for over thirty-six years that the application of a tensile overload can retard subsequent crack growth rates (Hudson and Hardrath, 1961). Considerable research effort has been expended on the subject of overload retardation since the early work of Hudson and Hardrath. Scores of journal papers have been published on the subject, resulting in dozens of analytical models, or model variants, on the prediction of overload effects. The success of some of these models has been inconsistent; they generally do a reasonable job of describing the observed results, but often lack the generality to be extended to other conditions. The objective of the present experiment was not to stop crack extension entirely; this could be accomplished merely with the application of a single massive tensile overload. The objective was, instead, to demonstrate that a more moderate degree of overload could retard crack growth rates (which had previously been at the EAC level) to a level such that diffusional mass transport could reduce the crack-tip sulfide level below that necessary to sustain EAC.

In spite of the variety of overload retardation models, virtually all models have a common feature: they all attempt to model the zone of residual compressive stresses at the crack tip and the attendant change in crack closure that results from the overload. This is illustrated schematically in Figure 2. Figure 2a shows the crack-tip plastic zone at the application of the tensile overload. Prior to the overload it is presumed that fatigue cycling was occurring at a lower level of $\sigma_{\text{max}}$; hence, a steady-state plastic zone smaller than the overload plastic zone that is pictured. Note that the stress distribution ahead of the crack tip in Figure 2a has the expected profile. Following the tensile overload, it is presumed that fatigue cycling returns to the earlier steady-state conditions. Figure 2b shows the stress distribution ahead of the crack tip following the tensile overload. Note that there is a relatively large zone of residual compressive stresses ahead of the crack tip. These residual stresses will partially, or wholly, offset the applied tensile stresses during the subsequent steady-state cycling. The magnitude of the overload, and the resulting zone of compressive stress will, of course, determine whether the subsequent crack growth rates will be partially or totally retarded. The opposite effect can also be operative: the application of a compressive underload can create tensile residual stresses at the crack tip which can locally accelerate crack growth rates (Rice and Stephens, 1973). This will be discussed in more detail later.

Figure 3 illustrates schematically the cyclic history of the experiment. With the exception of the planned overloads, fatigue cycling proceeded under the "steady-state" constant-$\Delta K$ conditions. The first single overload to 10-percent over $K_{\text{max}}$ occurred at 5,385 cycles. Steady-state cycling then resumed until another single overload to 20-percent over $K_{\text{max}}$ was applied at a total cycle count of 15,730 cycles. Steady-state cycling then resumed again until another single overload was applied at a total cycle count of 25,718 cycles to 40-percent over $K_{\text{max}}$. Steady-state cycling then resumed following the last overload, and continued until the end of the experiment at a total of 38,790 cycles. The crack length versus cycles results are plotted in Figure 4.

As discussed earlier, EAC readily occurs in this heat of A508-2 steel when tested under conditions conducive to EAC. This is illustrated in Figure 5 where earlier results from James (1994) are plotted in terms of the time-domain format originally introduced by Shoji et al. (1981, 1983). In the time domain format, the ordinate is the
experimentally-observed FCP rate \((\text{da/dN})\) divided by the load-rise time, while the abscissa is the mean FCP rate expected in an air environment at that value of \(\Delta K\) and \(R\) (Eason et al., 1989) divided by the load-rise time.

Test Phase I of the present study was conducted under the "Steady-State" conditions, and as will be seen in Figure 6, EAC rates were observed that were in good agreement with the earlier results in Figure 5. The application of a 10-percent overload resulted in only a mildly-retarded FCP rate (Figure 4), and as shown in Figure 6, Test Phase 2 continued under EAC conditions. The application of a 20-percent overload produced a somewhat greater FCP retardation in Test Phase 3a (Figure 4), and Figure 6 shows that the FCP rate was intermediate to the EAC and non-EAC extremes. However, although retarded, the average crack velocity during Test Phase 3a was \(V = 5.59 \times 10^{-7}\) mm/second. The average depth (from the notch) at this point was 17.65 mm, and per Equation [1a] the EAC Initiation Velocity would be expected to be \(V_{in} = 7.20 \times 10^{-8}\) mm/second. Hence, since the average crack velocity for Test Phase 3a exceeded the EAC Initiation Velocity by a factor of approximately 7.8, the retardation observed during Test Phase 3a would be expected to be temporary, and EAC would be expected to resume. This is indeed what happened during Test Phase 3b, and as may be seen in Figure 6, a "full" case of EAC resumed. The application of a 40-percent overload produced a significant degree of retardation in the FCP rates as shown in Figures 4 and 6. Note that crack extension had not ceased entirely, but that it was proceeding at such a low rate that is entirely likely that diffusional mass transport would reduce crack-tip sulfide concentration to a value less than the critical concentration (Wire and Li, 1996). During period of retardation, the FCP rates would gradually increase and approach the air or non-EAC levels only as the crack extended beyond the overload plastic zone. Any subsequent initiation of EAC would depend upon whether the subsequent average crack velocity exceeded \(V_{in}\); e.g., the criterion of Equation [1].

Because steady-state FCP rates cannot resume until the crack extends beyond the overload plastic zone, the present experiment was designed to provide several plastic zone diameters between each overload. Mills et al. (1977) found that, for ASTM A514-F steel, a crack extension of about two plastic zone diameters (based on a Dugdale yield model, Dugdale, 1960) was required between overloads to minimize overload interaction.

The crack-tip plastic zone radius \(r_p\) is a function of the applied K-level and the material yield strength \((\sigma_y)\). Table II shows that the room temperature yield strength of this heat of material is 468 MPa. The value of \(\sigma_y\) for this heat at 243°C is not known, but results for similar steels suggest that \(\sigma_y\) at 243°C should be about 90-percent of the room temperature value; hence, a value of \(\sigma_y = 421\) MPa will be used for 243°C.

The simple Irwin (1968) plastic zone radius (smaller than the Dugdale plastic zone used in Mills et al. (1977), but perhaps better suited for the present class of steel) can be expressed as

\[
r_p = \frac{1}{2\pi} \left( \frac{K_{max}}{\sigma_y} \right)^2 \tag{Eq. [2]}
\]

for "plane stress" conditions, and

\[
r_p = \frac{1}{6\pi} \left( \frac{K_{max}}{\sigma_y} \right)^2 \tag{Eq. [3]}
\]

for "plane strain" conditions. Since typical test specimens are generally not in a stress state corresponding to either of the above extremes, the crack-tip plastic zone radius is often estimated using an intermediate approximation:
Based on Equation [4], and employing $\sigma_y = 421$ MPa, the plastic zone radii for the 10%, 20%, and 40% overloads applied during this experiment are estimated to be 0.254, 0.305, and 0.432 mm, respectively. Mills et al. (1977) found that a crack extension of about two Dugdale yield zones (approximately $4r_p$ per Eq. [4]) was required between overloads to minimize overload interaction. It will be noted in Figure 4 that, with the exception of Test Phase 4, the crack was extended several plastic zone sizes between each overload.

CONCLUDING REMARKS

The present experiment clearly demonstrates that mitigation of an on-going case of EAC is possible with the application of a tensile overload of reasonable magnitude. Whether or not EAC subsequently reinitiates depends entirely upon the subsequent load cycling history. If, for example, EAC was operative and the fatigue crack growth rates had been temporarily retarded by the application of a tensile overload of the 40-percent magnitude in the present experiment, EAC might never re-initiate if the subsequent loading history included a significant period of cyclic inactivity (e.g., taking the unit out of service for a while) or subsequent operation involving less frequent loadings.

Although it is a useful demonstration of how a tensile overload can lead to mitigation of EAC, the present experiment is probably overly simplistic relative to more realistic loading histories. Actual loading histories could be considerably more complex than the simple loading schemes employed in this demonstration experiment.* Single tensile overloads were employed in the present experiment. Multiple tensile overloads applied consecutively have a greater crack growth retardation effect than single tensile overloads (Mills et al., 1977). Retardation is even greater when the multiple tensile overloads are separated by some critical amount of crack extension (Mills and Hertzberg, 1976). In addition, when a single tensile overload is directly followed by a single compressive underload, the subsequent crack growth retardation is minimized (although not eliminated) (Mills and Hertzberg, 1976). On the other hand, when only compressive underloads are applied, subsequent crack growth rates are actually temporarily accelerated until the crack reaches the boundary of the underload-affected plastic zone (Rice and Stephens, 1973). This can be thought of as the inverse of the situation pictured in Figure 2; a zone of residual tensile stresses due to the compressive underload results in temporarily higher local crack growth rates. Ignoring compressive underloads could result in non-conservative predictions of crack extension.

In spite of the simplistic nature of this experiment, it does provide additional insight into why the phenomenon of environmentally-assisted cracking is apparently not widely observed in many operating systems.

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*Variable-amplitude loading experiments have been conducted on medium-sulfur steels in a PWR environment at 288°C (Bamford, 1988; Cullen and Broek, 1989). The loading spectrum was designed to be at least partially prototypic of a commercial PWR plant. However, since EAC was not observed in this medium-sulfur steel under constant load-amplitude loadings, the authors did not have to deal with the additional modeling complications of time-dependent mass transport.
ACKNOWLEDGEMENTS

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REFERENCES


*Available from US DOE Office of Scientific Technical Information, P. O. Box 62, Oak Ridge, TN 37831


Table I
Chemical Composition (% weight) of Heat 15812

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<th>C</th>
<th>Mn</th>
<th>P</th>
<th>S</th>
<th>Si</th>
<th>Mo</th>
<th>Cu</th>
<th>Ni</th>
<th>Cr</th>
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<td>0.76</td>
<td>0.005</td>
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<td>0.62</td>
<td>-</td>
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<td>0.38</td>
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<td>0.16</td>
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Table II
Room Temperature Mechanical Properties of Heat 15812 (from VanDerSluys, 1993)

<table>
<thead>
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<th>0.2% Yield Strength</th>
<th>Ultimate Strength</th>
<th>Elongation</th>
<th>Reduction of Area</th>
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<tr>
<td>67.9 ksi (468 MPa)</td>
<td>85.6 ksi (590 MPa)</td>
<td>27.0%</td>
<td>56.2%</td>
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Figure 1. Electrochemical corrosion potential measurements (versus SHE) made throughout the experiment.

Figure 2. Schematic of the residual compressive stresses at a crack tip as a result of a tensile overload.
Figure 3. Schematic illustrating the application of tensile overloads. "Steady-state" cycling conditions resumed following each overload.

Figure 4. Crack length versus cycles plot showing the effect of tensile overloads.
Figure 5. Time-domain plot showing the EAC behavior of Heat 15812 in a 243°F low-oxygen environment (from James, 1994).

Figure 6. Time-domain plot of the crack growth response to various applied tensile overloads. See Figure 4 for a definition of the test phases.